# **Babcock and Mendenhall Safety Improvements**

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Placemaking

Infrastructure Engineering

Surveying + Mapping

Community Planning

Landscape Architecture

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#### INTRODUCTION

The Downtown Bozeman Transportation Study was initiated in 2020 with Part I being completed in December 2020. It evaluated design concepts for the recommended improvements in the 2019 Downtown Bozeman Improvement Plan (DBIP) for the following downtown streets: Babcock Street, Main Street and Mendenhall Street.

Sanderson Stewart is currently contracted with the Downtown Bozeman Partnership to complete the Babcock & Mendenhall Safety Improvements Study, which consists of data analysis and a traffic safety study focusing on corridor and intersection improvements along Babcock Street and Mendenhall Street. This report will provide a summary of the data analysis and safety solutions for the corridors and intersections.

#### SITE LOCATION AND DESCRIPTION

A study was performed to evaluate the downtown Bozeman operational and safety existing conditions and concerns, specifically for the corridors of Mendenhall Street and Babcock Street, between Tracy Avenue and Black Avenue. The corridors were studied, and a sign inventory was performed to help determine existing conditions for evaluation and recommended improvements. The Traffic volumes were collected and evaluated to determine the AM, noon and PM peak hours of travel which correlated with 8:30-9:30 AM, 11:45-12:45 PM, and 4:45-5:45 PM, respectively. Intersection capacity calculations were performed to determine that all intersections operate at an acceptable level of service, a qualitative measure of performance of an intersection abased on speed and travel time, freedom to maneuver and comfort and convenience. No vehicular traffic signal warrants were met.

A speed study was performed to determine travel speeds along the corridors. It was found that 85 percent of traffic travels between 27 and 30 mph on both corridors of Mendenhall Street and Babcock Street. Pedestrian and bicycle volumes were collected and evaluated to determine the AM, noon, and PM peak hours of travel. There was no clue morning peak hour for pedestrians/bicyclists, but a noon peak hour of 12:15-1:15 PM was evident with a smaller peak at 2:15-3:15 PM. Pedestrian level of service, which measures connectivity, directness and safety were found to be acceptable on Mendenhall Street, Main Street and Babcock Street.

#### **EXECUTIVE SUMMARY**

A crash analysis was performed to determine crash trends regarding frequency and severity at intersections and along the corridors. A historical crash rate higher than predicted was found at Mendenhall Street/Tracy Avenue. It was determined that pedestrian crashes typically resulted in injuries. There were four (4) pedestrian crashes in the study area, and several rear-end collisions resulting from a lead vehicle stopping for a pedestrian. Corridor crashes typically involved right angle crashes involving vehicles leaving access points, rear-end collisions and pedestrian/bicycle collisions, with the latter resulting in the most severe crashes.

A summary of FHWA's pedestrian safety countermeasures was provided. For roadways in the project area, per FHWA guidance, it is suggested that high visibility crosswalk markings, parking restrictions on crosswalk approaches, adequate nighttime lighting level and pedestrian crossing warning signs always be considered. Additionally, consideration should be given to raised crosswalks, in-street pedestrian crossing signs, and curb extensions. Each project intersection was evaluated for presence of the suggested countermeasures, and two (2) study area intersections, Tracy



Avenue/Mendenhall and Babcock Street/Black Street were used as example intersections. Graphics to illustrate the application of potential countermeasures and costs were provided. Recommendations to the study area were provided to help improve safety in downtown Bozeman, including the installation of curb bulb outs/extensions, increased crosswalk signs and pavement markings, lighting and restricting parking.

#### **EXISTING CONDITIONS**

#### Streets

Babcock Street (eastbound) and Mendenhall Street (westbound) are both one-way streets with two (2) travel lanes, Main Street has a four-lane section with two travel lanes in each direction, and the north/south streets through downtown generally have two-lane sections with two-way traffic. All downtown streets generally provide parallel parking on both sides of the street and there is consistent sidewalk throughout the project area.

There are painted striped crosswalks across all legs of every intersection in the downtown area. Pedestrian crossings at Main Street intersections are controlled by traffic signals. There are curb bulb-outs on some corners for pedestrian crossings along Babcock Street, but no other permanent pedestrian facilities currently exist in the study area. Temporary painted curb bulb-outs with delineators were installed during the summer of 2022 at the Babcock Street/Black Avenue intersection.

Mendenhall Street and Babcock Street have transit stops, and on Mendenhall Street there is a transit slip lane between the Tracy Avenue and Black Avenue cross streets, allowing buses to stop in a protected area and providing passenger safety without impacting traffic negatively.

#### Sign Inventory

A sign inventory was performed for the study area and details all signs on the corridors of Mendenhall Street and Babcock Street between North 5th Avenue and Willson Avenue. The inventory is provided for knowledge regarding existing signs, to coordinate wayfinding information and to help consolidate signage in the downtown area. Appendix A includes Exhibits A and B that detail the location of signs on the two corridors.

#### Traffic & Pedestrian Data and Analysis

Weekday AM, noon, and PM peak hour vehicle turning movement and bike/pedestrian counts were collected for study area intersections on Wednesday, July 13, 2022. The following study area intersections were evaluated because they are the heaviest traveled corridors located in the heart of downtown, where there is a focus on redevelopment and the most potential for traffic modifications such as traffic signals and bicycle corridors:

- Mendenhall Street/Black Avenue
- Mendenhall Street/Tracy Avenue
- Main Street/Black Avenue
- Main Street/Tracy Avenue
- Babcock Street/Black Avenue



#### Babcock Street/Tracy Avenue

The traffic data was collected using Miovision Scout video-based systems. In general, the weekday AM, noon, and PM vehicle peak hour periods were found to occur from 8:30-9:30 AM, 11:45 AM-12:45 PM, and 4:45-5:45 PM. Raw count data was adjusted for seasonal variation using MDT seasonal adjustment factors. Figure 1 (page 4) summarizes the Existing Conditions (2022) peak hour turning movement volumes for the AM, noon, and PM peak hours. Bike and pedestrian peak hours were slightly different than the vehicle peak hours, with no clear peak in the morning and the highest-volume hour of 12:15-1:15 PM, with a second smaller peak from 2:15-3:15 PM. Figure 2 (page 5) shows Existing Conditions (2022) bike/pedestrian counts for those peak hours, as well as AM volumes during the vehicle peak hour of 8:30-9:30 AM. Detailed vehicle traffic and bike/pedestrian count data worksheets are included in Appendix B.

Truck traffic was observed during the peak hours previously mentioned. During the AM, noon and PM peak hours, truck traffic on Babcock Street compromised 1.8%, 1.4% and 1.5% of all traffic, respectively. During the AM, noon and PM peak hours on Mendenhall Street, truck traffic was observed to be 5.2%, 2.7% and 2.0% of traffic, respectively. On Main Street, during the AM, noon and PM peak hours, trucks were 2.8%/3.0% (eastbound/westbound), 2.5%/4.0% (eastbound/westbound), and 1.6%/0.7% (eastbound/westbound) of traffic, respectively. Truck traffic on Tracy Avenue at Mendenhall Street was highest on the southbound approaches during all peak hours, with the AM peak, noon and PM peak hours making up 15.4%, 9.7% and 5.1% of all traffic, respectively.

#### Couplet Roadways & Conversions

Babcock Street and Mendenhall Street are couplets serving downtown Bozeman. Couplets are two parallel one-way roadways that run in opposite directions to provide two-way mobility in a specific area and typically connect to roadways with two-way traffic. They are established to provide greater capacity for vehicles and provide an opportunity to reduce pedestrian crossing distances by adding curb extensions or narrowing travel lanes. They typically have fewer intersection turning movements that may increase safety and improve pedestrian crossing safety as vehicles are approaching from one less direction. They also provide an opportunity to create bicycle lanes and/or, in Bozeman's context, on-street parking.

Disadvantages of couplets include increased travel time and out of direction travel for local residents, delays for emergency vehicles, and without appropriate traffic management strategies, they may provide context for increased vehicle speeds. One-way streets are typically associated with higher speeds and multi-lane one-way streets create a multiple threat condition for pedestrians crossing a road. Additionally, left-turning motor vehicle drivers may be less cautious when turning from a one-way street, and therefore less inclined to notice a pedestrian.

In terms of pedestrian safety, there are benefits to both one-way and two-way streets, and so the decision to convert a one-way street to two-way is context sensitive. FHWA highlighted one-way/two-way street conversions in a case study, PedSafe 2013, Pedestrian Safety Guide and Countermeasures Selection System, and suggested one-way streets should have traffic calming features to reduce vehicle speeds and design features, such as curb extensions and turning radius reductions, to discourage high vehicular speeds.

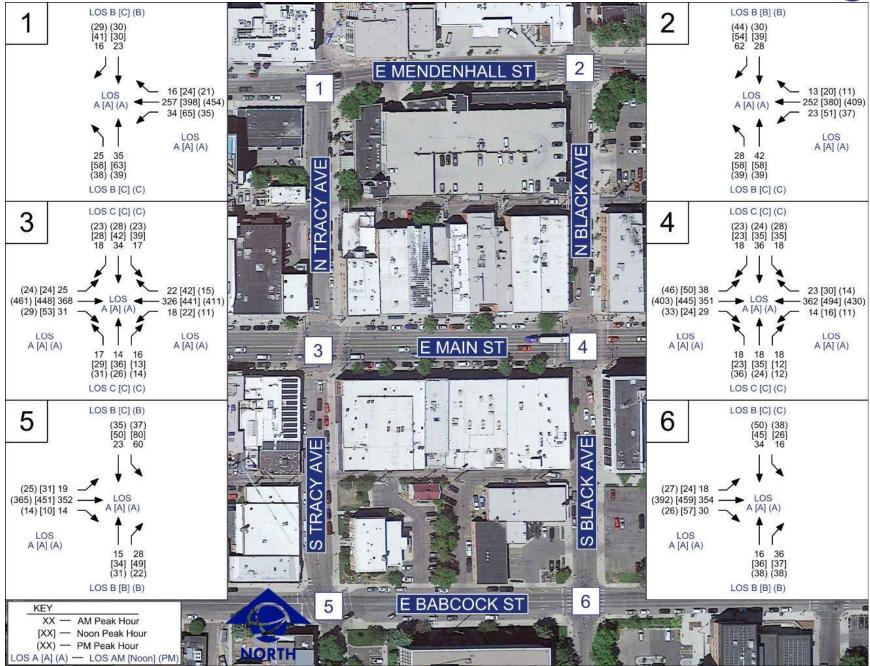


Figure I: Existing Conditions (2022) Vehicle Turning Movements for Peak Hours

Downtown Bozeman Safety Study



Figure 2: Existing Conditions (2022) Pedestrian/Bicycle Turning Movements for Peak Hours Downtown Bozeman Safety Study



#### **Parking**

Main Street has on-street parallel parking on both sides of the street throughout the project corridor, with parking at some intersections less than 20 feet from the crosswalks. Babcock Street has on-street parallel parking on the south side throughout the project area and on the north side of the roadway east of Bozeman Avenue and west of Black Avenue. Parking extends to approximately 30 feet from crosswalks, and the intersection with Tracy Avenue has bulb-outs on each corner with the exception of the southwest corner. Mendenhall Street has parallel parking on both sides of the street throughout the project area with the exception of the block between Black Avenue and Tracy Avenue, where a transit island is present. The distance of on-street parking on Mendenhall Street varies but is typically approximately 20 feet from the crosswalks. There are several locations in downtown Bozeman where trucks double park in traffic lanes for unloading and/or loading. Consideration should be given to providing a 15-minute loading/unloading space in areas that have frequent double-parked trucks that are unloading/loading at local businesses.

#### On-Street Bicycle Lanes

Under existing conditions there are no on-street bicycle lanes in the downtown study area. Some roadways have "sharrows," pavement markings designating that vehicles and cyclists share the road. In the summer the City provides bicycle racks located in on-street parking stalls to encourage bicycling. Consideration should be given to consistently painting sharrows on downtown roads to bring awareness to bicyclists and drivers that they are sharing the roadways.

In the 2019 DBIP study, one concept was to remove the north side on-street parking on Babcock Street and replace it with a two-way cycle track to provide cyclists a separated space instead of sharing the roadway and/or sidewalks. Narrowed drive lanes were suggested to provide traffic calming to lower speeds and allow room for the cycle track. The report also recognized that providing a cycle track was a challenge with the width constraints on Babcock Street the presence of numerous driveway egresses, the existing traffic control at intersections which presently is for eastbound only traffic and would result in the loss of 81 on-street parking spaces. Due to the mentioned challenges, sharrows were recommended in both lanes for Babcock Street and Mendenhall Street in the 2019 DBIP, and in the southern lane of Mendenhall Street and as a secondary option on Babcock Street (to create a cycle track) in the Phase I Downtown Bozeman study. Consideration should be given to applying sharrow pavement markings uniformly throughout downtown.

#### Vehicle Intersection Capacity

Existing Conditions (2022) vehicle intersection capacity calculations were performed for the study area intersections using Synchro, Version 11, which is based on the Highway Capacity Manual, 6th Edition (Transportation Research Board, 2016). Level of service (LOS) is defined as a quality measure describing operational conditions within a traffic stream, generally in terms of such service measures as speed and travel time, freedom to maneuver, traffic interruptions, comfort, and convenience. LOS is a qualitative measure of the performance of an intersection with values ranging from LOS A, which indicates good operation and low vehicle delays, to LOS F, which indicates congestion and longer vehicle delays. LOS C is typically considered a minimum acceptable threshold for operations in Montana-based communities, though exceptions are made in certain cases.

The results of the Existing Conditions (2022) vehicle intersection capacity calculations showed that all intersections and approaches operate at LOS C or better during all three peak hours. Projected 95th percentile queues are minimal on all



approaches. Figure I also shows the Existing Conditions (2022) LOS results at each intersection. A detailed capacity summary table and capacity calculation worksheets for each of the study area intersections can be found in Appendix C.

#### Signal Warrant Analysis

Traffic signal warrants were evaluated at the four (4) stop-controlled intersections of Mendenhall Street and Babcock Street with Tracy Avenue and Black Avenue using criteria outlined in the Manual on Uniform Traffic Control Devices (MUTCD) for the Existing Conditions (2022) vehicular traffic volume scenario. The MUTCD presents several warrants that can be considered based on traffic volumes, school crossings, crash history, and others. For the purposes of this analysis, all but Warrants 5 and 9 (School Crossing and Intersection Near a Railroad Grade Crossing) were evaluated because there are no school or railroad crossings near the intersections. Satisfaction of the Peak Hour warrant alone should not be considered as warranting a signal, as it is primarily meant for application at office complexes, manufacturing plants, or other high-occupancy vehicle facilities that attract or discharge large numbers of vehicles over a short time.

Signal warrants are not close to being met at any of the four (4) intersections when evaluated with Existing Conditions (2022) volumes. Traffic signal warrant worksheets can be found in Appendix D.

#### Speed Data

Speed data was gathered using StatTrak Data Collectors and Houston Radar Stats Analyzer on July 11-15, 2022, on Babcock Street between Black Avenue and Bozeman Avenue, Babcock Street between South 3rd Avenue and South Grand Avenue, Mendenhall Street between North 3rd Avenue and North Grand Avenue, and Mendenhall Street between North Bozeman Avenue and Rouse Avenue. Posted speeds on all roadways are 25 mph. 50th percentile and 85th percentile speeds, the speeds that 50 percent of traffic and 85 percent of traffic are observed to travel at or below during free-flow conditions, were collected in addition to maximum speeds and are illustrated in Table 1 below.

Table I: Speed Data

	Mendenhall -	Mendenhall -	Babcock -	Babcock -
	3rd & Grand	Bozeman & Rouse	Black & Bozeman	3rd & Grand
Date	7/11-13/2022	7/13-15/2022	7/13-15/2022	7/11-13/2022
Direction of Vehicles	Westbound	Westbound	Eastbound	Eastbound
ADT (veh)	5,148	4,355	5,774	4,497
85th Percentile Speed (mph)	30	27	28	30
Max speed (mph)	49	55	49	48
50th Percentile Speed (mph)	26	23	24	26

85th percentile speeds are used to help establish appropriate speed limits. It should be noted that the radar unit on Mendenhall Street between 3rd Avenue and Grand Avenue had sporadic high speeds (8 data points over 70 mph), so that data was manually removed from the analysis. Speed data summary sheets can be found in Appendix E.



Eighty-fifth (85th) percentile speeds collected on Mendenhall Street and Babcock Street exceeded the posted speed limit of 25 mph, with 85th percentile speeds being 27 mph and 28 mph within the immediate downtown area (Bozeman Avenue and Rouse Avenue) and slightly higher (30 mph) further outside of the center of downtown (3rd Avenue and Grand Avenue).

#### Pedestrian Level of Service

Motor vehicles on roadways have a level of service (LOS) that is a quality measure describing operational conditions within a traffic stream, generally in terms of such service measures as speed and travel time, freedom to manuever, traffic interuptions, and comfort and convenience. Similarly, an algorithm to determine pedestrian LOS has been created with considerations including a combination of connectivity, directness, safety and capacity measures. Table 2 below presents the pedestrian LOS for the downtown intersections on Mendenhall Street and Babcock Street. Generally, pedestrians do not have a long wait before being able to cross with either a break in traffic or vehicles yielding to pedestrians. The LOS was higher on Main Street, most likely due to signalized pedestrian crossings providing more protection.

**Table 2: Pedestrian Level of Service** 

Road Segment	Pedestrian Level of Service
Mendenhall	2.05 (B)
Tracy to Black	2.03 (b)
Main	1.25 (A)
Tracy to Black	1.23 (A)
Babcock	1 40 (D)
Tracy to Black	1.68 (B)

#### **CRASH HISTORY**

Crashes for corridor segements and study area intersections were evaluated to determine any trends with regards to frequency or severity and potential safety treatments.

#### Intersection Crash History

Historical intersection and corridor crash data was obtained from MDT for the 5-year period from January 1, 2014, through December 31, 2018. Crash data was collected from the City of Bozeman Police Department from January 1, 2019 through October 2022 and evaluated separately. The data was analyzed for the purposes of calculating intersection crash and severity rates and evaluating corridor and intersection collision type trends. Tables 3 (page 9) and 4 (page 10) illustrate the results of the historical crash analysis.

Intersection crash rates were calculated on the standard basis of crashes per million vehicles entering (MVE) for each intersection. The MVE metric was estimated based on 2022 peak hour traffic counts and published historical ADT



volumes from the MDT website. Crash rates for the study area intersections ranged from 0.17 crashes/MVE to 0.86 crashes/MVE.

As a means of evaluating the historical crash frequency rates, Sanderson Stewart calculated expected rates using the predictive crash rate formulas in the American Association of State Highway Transportation Officials (AASHTO) Highway Safety Manual (HSM). The process involves calculating the number of crashes predicted in a year based on traffic demand (AADTs) and various physical and traffic environment-based conditions such as lane configurations, traffic signal phasing, and approach speeds. The calculations result in a crashes-per-year prediction. Sanderson Stewart then back calculated a frequency rate on the basis of MVE for the sake of comparison with the actual historical crash rate. The results of the calculations for this study showed that the historical crash rate was slightly higher than the predicted crash rate at the intersection of Mendenhall Street/Tracy Avenue. The historical crash rates were lower than the predicted crash rates at all other study area intersections. The HSM rate predictions and 5-year crash totals for each intersection are summarized in Table 3 below.

Table 3: Crash Frequency and Severity, 2014-2018

		Crash Type			Crash Data <sup>3</sup>			HSM Predictions⁴		
Intersection	2014-2018 DEV <sup>1</sup>	I-2018 Reported EV <sup>1</sup> Crashes <sup>2</sup>		Injury	Fatality	Average Crash Frequency (Crash/Yr)	Crash Rate (Crash/ MVE)	Severity Index	Predicted Average Crash Frequency (Crash/Yr)	Predicted Crash Rate (Crash/ MVE)
Mendenhall St/Black Ave	3078	I	0	I	0	0.20	0.18	3.00	0.94	0.84
Mendenhall St/Tracy Ave	3178	5	4	I	0	1.00	0.86	1.40	0.92	0.79
Main St/Black Ave	13097	4	2	2	0	0.80	0.17	2.00	1.49	0.31
Main St/Tracy Ave	15095	5	3	2	0	1.00	0.18	1.80	2.72	0.49
Babcock St/Black Ave	4029	2	2	0	0	0.40	0.27	1.00	1.14	0.78
Babcock St/Tracy Ave	3853	4	3	I	0	0.80	0.57	1.50	1.16	0.82

<sup>&</sup>lt;sup>T</sup> Daily Entering Volume (DEV) estimated from 2022 peak hour counts and 2014 through 2018 MDT published ADTs

Severity index is defined as the weighted average by crash severity, including fatality, injury, and property damage only (PDO) crashes. The highest severity index was 3.00 at the intersection of Mendenhall Street/Black Avenue due to the only crash resulting in injuries. At the intersection of Main Street/Black Avenue the severity index was 2.00, due to 2 of 4 (50%) of crashes resulting in injuries. Overall, pedestrian crashes typically resulted in injuries, with 2 of 4 (50%) pedestrian crashes resulting in severe injuries. The two severe injury related pedestrian crashes occurred at the intersections of Babcock Street/Tracy Avenue and Mendenhall Street/Black Avenue with dry pavement and clear weather conditions, and the latter pedestrian crash occurred in the dark. Severity index calculation results are also shown in Table 3.

Sanderson Stewart performed an analysis of intersection collision classification to determine if any patterns could be identified. Table 4 on page 10 illustrates the results of the types of crashes in the study area. There is a history of rearend crashes on the Main Street intersections and the intersection of Mendenhall Street/Tracy Avenue. The intersection of Mendenhall Street/Tracy Avenue also had 2 of 5 (40%) of crashes due to right-angle collisions. The intersections

<sup>&</sup>lt;sup>2</sup> Crashes reported from January 1, 2014 to December 31, 2018

<sup>&</sup>lt;sup>3</sup> Crash rates expressed as crashes per million vehicles entering (MVE)

<sup>&</sup>lt;sup>4</sup> Rates calculated using Highway Safety Manual (HSM) 1st Edition predictive methodology using SPICE tool



along Babcock Street and Main Street each had I sideswipe, same direction crash. There were 4 pedestrian crashes at the six (6) intersections, including one at each of the following intersections: Babcock Street/Tracy Avenue, Main Street/Tracy Avenue and Mendenhall Street/Black Avenue.

Table 4: Intersection Crash History - Collision Type, 2014-2018

		Collision Type								
	Rear	Right								
	End	Angle	LT, SD	SS, SD	Pedestrian	Other	Total			
Mendenhall St/Black Ave					I		I			
Mendenhall St/Tracy Ave	2	2	I				5			
Main St/Black Ave	2			ļ	I		4			
Main St/Tracy Ave	2	I		l	I		5			
Babcock St/Black Ave		I		l			2			
Babcock St/Tracy Ave		I		Ī	Ī	Ī	4			

<sup>\*</sup>Crashes reported from January 1, 2014 to December 31, 2018

Rear-end collisions commonly occur at signalized intersections because the signalized control is dynamic, and thereby requires drivers to recognize and react to changing conditions in real time. High speeds approaching signalized intersections allow drivers less response time, which creates an opportunity for an increased chance of rear-end collisions. Another potential contributing factor for rear-end collisions at a signalized intersection is a yellow change interval that is too short for the prevailing operating speeds in that corridor.

Based on the application of yellow change interval formulas from the Institute of Transportation Engineers (ITE) Traffic Engineering Handbook with assumed 25 mph thru-movement operating speeds, it was calculated that the minimum "design" yellow change intervals of 3.0 seconds for the approaches are adequate. For intersections where the 85th percentile speeds are closer to 30 mph, then the 3.0 second yellow clearance may be inadequate. Consideration should be given to increasing the yellow clearances to 3.2 seconds to conform to ITE recommendations that more closely match the 85th percentile speeds of 30 mph.

The unsignalized intersection of Mendenhall Street/Tracy Avenue had 2 of 5 (40%) of crashes due to rear-end collisions, and 2 of 5 (40%) due to right-angle collisions. Both rear-end collisions involved westbound vehicles. One (I) rear-end collision occurred when a westbound vehicle hit a parked vehicle in snowy conditions. The other rear-end collision occurred when a westbound vehicle failed to yield to the vehicle in front of it during dark, lighted conditions with dry pavement and clear weather. More information would need to be provided to determine exact crash causes.

Right-angle collisions often occur at unsignalized intersections after a driver stops at a stop sign and then proceeds when it is unsafe to do so due to limited sight distance or inadequately judging gaps in vehicles. At the intersection of Mendenhall Street/Tracy Avenue both crashes involved a westbound vehicle colliding with a southbound vehicle. It may be possible that higher speeds on Mendenhall Street make it difficult for minor road drivers to judge vehicle gaps. Increasing visibility for the intersection on the minor roadway may also help to improve safety. More visible pavement markings and the application of a larger stop sign on Tracy Avenue may call more attention to the intersection to help increase safety. Although not necessarily the cause of right-angle collisions with southbound vehicles, on the south approach of this intersection it is possible that sight distance may be prohibited to the east by the trees located on the



southeast quadrant of the intersection. Consideration should be given to maintaining and pruning the trees to help improve sight distance and improve safety.

Sideswipe, same direction crashes typically occur at intersections when there are not dedicated turn lanes and when vehicles switch lanes to avoid slowing or queued vehicles approaching a turning movement. At the unsignalized intersection of Babcock Street/Tracy Avenue, one (I) sideswipe, same direction crash occurred due to an eastbound vehicle colliding with a vehicle that was changing lanes. The other sideswipe crash at Babcock Street/Black Avenue involved an eastbound vehicle hitting a parked car during clear daylight conditions. More detailed crash data would be needed to determine the exact causes of the crashes.

The intersections of Main Street/Black Avenue and Main Street/Tracy Avenue had one (I) sideswipe, same direction crash. The crashes at both Main Street/Black Avenue and Main Street/Tracy Avenue involved westbound parked vehicles entering the traffic lane and being sideswiped by a passing vehicle.

The four (4) pedestrian crashes occurring at the study area intersections (Babcock Street/Tracy Avenue, Main Street/Black Avenue, Main Street/Tracy Avenue and Mendenhall Street/Black Avenue) typically occurred with clear weather conditions and dry pavement and all crashes resulted in injuries. Two (2) crashes involved vehicles making a left-turn and the other two (2) crashes involved vehicles traveling straight before hitting the pedestrian. More crash information would need to be provided to determine exact causes and potential safety countermeasures, however enhanced pedestrian crossings and lower vehicle speeds through the high pedestrian traveled intersections may help improve pedestrian safety.

Crash data was collected from the City of Bozeman Police Department for the period of January 1, 2019 through October 21, 2022 for the intersections of Babcock Street/Tracy Avenue, Babcock Street/Black Avenue, Mendenhall Street/Tracy Avenue and Mendenhall Street/Black Avenue. Collision types for corridor crashes are provided in Table 5 below:

Table 5: Intersection Crash History -- Collision Type, 2019-2022

			n Type			
Intersection	SS, SD	LT, SD	Rear End	Right Angle	Fixed Object	Pedestrian
Mendenhall Street/	2	4	1	Ī	1	ı
Black Avenue	2	т	I I	I.	т	I.
Mendenhall Street/	_		ı	2	ı	
Tracy Ave	Į.		1	2	•	
Babcock Street/	2	ı	ī	Е	Ī	
Black Avenue	2	<sup>2</sup>   '	1	3	I.	
Babcock Street/	4	2	2	ī	1	
Tracy Ave	4	3		I	1	

<sup>\*</sup>Crashes reported from January 1, 2019 to October 21, 2022



Common crash types were sideswipe, same direction crashes that occurred when vehicles were switching lanes and failed to yield to a vehicle in the adjacent lane. Left-turn, same direction crashes commonly occurred when vehicles were making a left-turn from the far travel lane on the one-way roadway and collided with a vehicle in the internal lane. There were several rear-end collisions, often occurring when the lead vehicles were stopping for a pedestrian, and several right-angle collisions involving a left-turning vehicle colliding with a through moving vehicle. One pedestrian crash was recorded. Crash severity was not available from this crash database.

#### Corridor Crash History

Corridor crash data was evaluated for the five-year period of January 1, 2014 to December 31, 2018 to determine trends along the downtown corridors of Babcock Street and Mendenhall Street between Willson Avenue and Rouse Avenue. Previously referenced Black Avenue and Tracy Avenue intersection-related crashes are included in Table 6 (below) which summarizes crash data, but will not be discussed in detail.

Babcock Street, between Willson Avenue and Rouse Avenue, had two (2) non-intersection related crashes. Both crashes (one coded as sideswipe, same direction and the other as a rear-end collision) involved a vehicle colliding with a parked car and resulted in no injuries. The remaining crashes occurred at intersections or accesses along Babcock Street. There were 2 of 24 (8%) crashes involving pedestrians/bicyclists and both resulted in injuries. Fifteen (15) of 24 (63%) crashes were right-angle collisions and 5 of 15 (33%) right-angle collisions resulted in injuries. The most severe crashes on Babcock Street were a pedestrian related crash with an eastbound vehicle near Tracy Avenue and a right-angle collision that resulted in a severe injury at Babcock Street/Willson Avenue. This crash involved an eastbound and a southbound vehicle.

Table 6: Corridor Crash History - Collision Type, 2014-2018

		-											
Corridor	Rear End	Fell from Vehicle	Fixed Object	LT, SD	SS,SD	Right Angle	Backing Vehicle	Pedestrian	Bicycle	Total			
Babcock Street	1			2	2	15		2		24			
S Willson Ave - S Rouse Ave	'	'	'	'	'			3	13		2		24
W Mendenhall Street	Е					2		1	2	15			
S Willson Ave - S Rouse Ave	5		'	'	'	3		' '	2	13			

<sup>\*</sup>Crashes reported from January 1, 2014 to December 31, 2018

On Mendenhall Street, between Willson Avenue and Rouse Avenue, there were two (2) crashes that were not intersection related. Both crashes involved vehicles colliding with parked vehicles; one crash was reported as a sideswipe, same direction and the other as a rear-end collision and both crashes resulted in property damage only. The remainder of the crashes along the corridor of Mendenhall Street occurred at intersections. There were two bicycle related crashes that occurred at the intersections of Mendenhall Street/Willson Avenue and Mendenhall Street/Bozeman Avenue. Both bicycle crashes resulted in injuries.

When evaluating crashes, it is important to consider both crash frequency and severity. Table 7 (page 13) provides the breakdown of crash severity for crashes along the Babcock Street and Mendenhall Street corridors. On Babcock Street, between Willson Avenue and Rouse Avenue, 17 of 24 (71%) crashes were property damage only with the remaining 29% resulting in various levels of severity. Mendenhall Street had 4 of 14 (29%) crashes resulting in some type of injury.

Table 7: Summary of Crash Severity for Corridor Crashes, 2014-2018

	Crash Severity							
Corridor	Property Damage Only	Possible Injury Crash	Suspected Minor Injury	Suspected Severe Injury				
Babcock Street	17	ı	4	2				
S Willson Ave - S Rouse Ave	17	l	7	2				
W Mendenhall Street	10	2	ı	1				
S Willson Ave - S Rouse Ave	10	2	l	ı				

<sup>\*</sup>Crashes reported from January 1, 2014 to December 31, 2018

The most frequent crash types that resulted in injuries are presented in Table 8 (below). On Babcock Street the most severe crashes were caused by right-angle and pedestrian collisions. Mendenhall Street had the most severe injury crashes because of pedestrian and bicycle collisions. Pedestrian and bicycle safety countermeasures may help reduce the severity of crashes.

Table 8: Most Frequent Cause of Corridor Crashes by Severity

Corridor	Possible Injury Crash	Suspected Minor Injury	Suspected Severe Injury	
Babcock St	Right Angle	Pedestrian	Pedestrian	
Dabcock St	Night Aligie	Right Angle (3)	Right Angle	
Mendenhall St	Rear End (2)	Bicycle	Pedestrian	
T Terraerman se	Bicycle	Dieyeie	r edescrian	

<sup>\*</sup>Crashes reported from January I, 2014 to December 31, 2018

It is important to note that all the previous evaluations are speculative, and more detailed information about individual crashes would be needed to determine exact causes for each collision.

#### Pedestrian Safety Countermeasures for Crash Reductions

The Federal Highway Administration (FHWA) has created a Best Practices Toolbox of countermeasures and their potential effectiveness to reduce pedestrian crashes. Signalization, geometric and operational countermeasures are provided in the Toolbox with their targeted crash type (fatality, injury, property damage only or all) and calculated crash reduction factor (CRF), the percentage crash reduction that might be expected after implementing a given countermeasure.

The countermeasures that may be applicable to Mendenhall Street, Main Street and Babcock Street are summarized in Table 9 on page 14. More specific information on applications in Bozeman follow in the report.

Table 9: Best Practices Toolbox of Countermeasures for Pedestrian Safety, FHWA

Countermeasure	Crash Severity	CRF for Pedestrian				
		Crashes (%)				
Signalization						
Add exclusive pedestrian phasing	All	34				
Replace Walk/Don't Walk Signals with	All	25				
Pedestrian Countdown Signal Heads	All	25				
Modify signal phasing (implement a leading	All	5				
pedestrian interval)	All	3				
Geometric Countermeasures						
Install Raised Pedestrian Crossing	All	30				
mistali Naised i edestriali Crossing	Fatal/Injury	36				
Signs/Markings/Open	erational Countermeasures					
	All	21				
Add Intersection Lighting	All	(nighttime crashes)				
Add intersection Lighting	Injury	27				
	Injury	(nighttime crashes)				
Prohibit Right Turn on Red		3				
Prohibit Left Turns		10				
Restrict on-street parking near	All	30				
intersections	All	30				

#### Uncontrolled Crossing Pedestrian Safety Countermeasures

FHWA's Best Practices Toolbox includes the *Guide for Improving Pedestrian Safety at Uncontrolled Crossing Locations* (2018) to address safety issues at locations where pedestrian facilities intersect with uncontrolled roadways. The guide helps assist with selecting appropriate countermeasures based on roadway volumes, posted speeds and lane configurations to improve crossing safety. For a two-lane roadway, with posted speeds of 30 mph or below with AADT of less than 9,000 vehicles per day, the following pedestrian crash countermeasures should always be implemented if feasible:

- High visibility crosswalk markings
- Parking restrictions on crosswalk approaches
- Adequate nighttime lighting levels
- Pedestrian crossing warning signs

Additionally, consideration should also be given to the following countermeasures:

- Raised crosswalks
- In-street pedestrian crossing sign
- Curb extensions



The evaluated countermeasures help address conflicts with crossing locations, vehicle speeds, inadequate conspicuity/visibility, drivers not yielding to pedestrians, and insufficient separation of pedestrians from traffic as shown in Table 10 (below) from the guide.

Table 10: Safety Issues Addressed Per Countermeasure, FHWA

	Safety Issue Addressed								
Pedestrian Crash Countermeasure for Uncontrolled Crossings	Conflicts at crossing locations	Excessive Vehicle Speed	Inadequate Conspicuity/ Visibility	Drivers not yielding to pedestrians in crosswalks	Insufficient separation from traffic				
Crosswalk Visibility Enhancement	斉	斉	片	斉	广				
High-visibility crosswalk markings*	序		片	序					
Parking restriction on crosswalk approach*	序		广	序					
Improved nighttime lighting*	序		序						
Advance yield here to pedestrian sign and yield line*	齐		序	齐	秀				
In-Street pedestrian crossing sign*	齐	<u> </u>	序	齐					
Curb Extension*	齐	<u> </u>	片		秀				
Raised Crosswalk	序	<u> </u>	广	序					
Pedestrian Refuge Island	序	序	广		· <b>广</b>				
Pedestrian Hybrid Beacon	序	序	广	序					
Road Diet	序	序	片		广				
Rectangular Rapid- Flashing Beacon	序		广	片	序				

<sup>\*</sup>These countermeasures make up the STEP Countermeasures. Crossing visibility enhancements and multiple countermeasures may be implemented at a location as part of crosswalk visibility enhancements.

#### PEDESTRIAN SAFETY COUNTERMEASURES

FHWA has compiled proven pedestrian safety countermeasures including crosswalk visibility enhancements; these enhancements include high visibility crosswalks, lighting, and signing and pavement markings to make crosswalks, and users, more visible to drivers.

#### High Visibility Crosswalk Markings

High visibility crosswalk markings use patterns (bar pairs, continental, ladder) that are visible to both the driver and pedestrian from farther away compared to traditional transverse line crosswalks. Both FHWA and the National Association of City Transportation Officials (NACTO) suggest the use of retroreflective pavement markings to improve safety. Retroreflective materials such as inlay or thermoplastic tape, instead of paint or brick, should be used for highly reflective crosswalk markings to improve safety, particularly at night. They should be provided at all established midblock pedestrian crossings and considered at all uncontrolled intersections. Their application is anticipated to reduce pedestrian injury crashes up to 40%.



#### Parking Restrictions on Crosswalk Approach

The FHWA acknowledges that on-street parking has an important relationship to pedestrian and motorist's safety. On-street parking can create a buffer, separating pedestrians from vehicular traffic on the adjacent roadway and reducing motorist's travel speeds. However, on-street parking also blocks visibility between the pedestrians and the motorists, particularly for children.

Parking restrictions can include the removal of parking spots close to an intersection, installation of new "parking prohibited" pavement markings or curb paint and signs. The suggested minimum setback is 20 feet in advance of a crosswalk where speeds are 25 mph or less. Thirty (30) foot setbacks are recommended for speeds between 26 and 35 mph, which would be more appropriate for the Downtown Bozeman intersections. The negative aspects of restricting curb parking are that it may eliminate a parking space for motorists, which may be opposed by nearby business owners, and potential increases in speeds for vehicles, which is also undesirable for pedestrians. Restricting on-street parking near intersections has been proven to reduce pedestrian crashes by 30%.

#### Adequate Nighttime Lighting Levels

To provide adequate nighttime lighting levels, consideration should be given to placing lights in advance of midblock and intersection crosswalks on both approaches to illuminate the front of the pedestrian and avoid creating a silhouette. Intersection lighting has been proven to reduce all pedestrian nighttime crashes by 21% and injury pedestrian nighttime crashes by 27%.

#### Crosswalk Warning Signs

Crosswalk warning signs are advanced crosswalk yield signs that are placed 20-50 feet in advance of a marked crosswalk to indicate where vehicles are required to yield to pedestrians. To supplement the signing, a yield bar (commonly referred to as "shark's teeth") pavement marking can be used to indicate where vehicles are required to yield. Advanced crosswalk signs are typically used when a crossing is unexpected, such as mid-block. They have shown a reduction of 25% in pedestrian crashes.

#### Raised Crosswalks

Raised crosswalks are ramped speed tables spanning the entire width of the roadway to make the pedestrian more prominent in the driver's field of vision. The crossing is marked with paint or special paving materials. These crosswalks may reduce vehicle speeds, improve motorist yielding and allow the pedestrian to cross at grade. The raised crosswalk is flush with the height of the sidewalk and is typically at least ten feet wide so a vehicle's front and back tires can be on the table at the same time. Drainage needs to be given careful consideration. Raised crosswalks may not be appropriate for primary emergency vehicle routes and can be a concern for snowplowing. They have shown a 45% reduction in pedestrian crashes.



#### In-Street Pedestrian Signs

In-street yield to pedestrian signing may be appropriate on 2- or 3-lane roads where speed limits are 30 mph or less. They serve to remind road users of laws regarding yielding to pedestrians and are typically placed in between travel lanes. They can be a concern for snowplowing due to their location in the middle of the roadway. There currently is no crash reduction factor calculated for in-street pedestrian signs.

#### **Curb Extensions**

An alternative to improve safety and maintain parking is to provide curb extensions, also referred to as bulb-outs, at pedestrian crossings. Curb extensions extend the sidewalk or curb line out into the parking lane, which reduces the street width. They shorten the distance for pedestrians to cross, allow the pedestrians to be more visible to motorists approaching intersections, and provide protection for pedestrians trying to view approaching vehicles instead of walking into a lane of traffic.

Curb extensions can improve accessibility for emergency vehicles as intersections are kept clear of parked cars. They can also be used to place landscaping or streetscaping. Curb extensions must not extend into travel lanes and should not extend into bicycle lanes. There currently is no crash reduction factor calculated for curb extensions.

#### City of Bozeman Pedestrian Safety Countermeasures

Mendenhall Street and Babcock Street were evaluated to determine which of the suggested countermeasures from FHWA's Guide for Improving Pedestrian Safety at Uncontrolled Crossing Locations are currently installed at the uncontrolled crossings of Bozeman Avenue, Black Avenue and Tracy Avenue. Table 11 below illustrates the countermeasures currently installed.

Table II: Pedestrian Countermeasures Currently in Use in Bozeman

	Intersections					
Treatment	Babcock/	Babcock/	Babcock/	Mendenhall/	Mendenhall	Mendenhall
	Bozeman	Black	Tracy	Bozeman	/Black	/Tracy
High Visibility Crosswalk Markings		✓	✓			
Parking Restrictions on Crosswalk Approach	✓		✓	✓	✓	✓
Adequate Nighttime Lighting Levels	✓			✓	✓	✓
Crosswalk Warning Signs		✓	✓			
Raised Crosswalks						
In-Street Pedestrian Crossing Sign						
Curb Extension		✓				



#### Application of Pedestrian Countermeasures

Sanderson Stewart met with the Downtown Bozeman Partnership staff on October 25, 2022 and discussed the feasibility of applying uniform pedestrian countermeasures at downtown intersections. To provide information on application and cost, the intersections of Mendenhall Street/Tracy Avenue and Babcock Street/Black Avenue were selected as examples. At the intersection of Mendenhall Street/Tracy Avenue the following countermeasures were evaluated: High Visibility Crosswalks, Crosswalk Warning Signs, In-Street Pedestrian Crossing Sign and Curb Extensions. The intersection of Babcock Street/Black Avenue was used to evaluate the following countermeasures: High Visibility Pavement Markings, Parking Restriction on Crosswalk Approach (via a curb bulb-out), Nighttime Lighting Levels, and In-Street Pedestrian Crossing Signs.

Figures 3 and 4 on pages 19 and 20 provide a bird's eye view and perspective image for the two intersections with the application of the discussed pedestrian countermeasures. Raised medians were not considered due to drainage and plowing concerns. Utility work was estimated, and prices are provided based upon recent construction projects in Bozeman. Table 12 below provides the anticipated crash reduction factor and estimated cost for implementation. Check marks are present in boxes where these treatments are presently installed.

Table 12: Pedestrian Safety Treatments and Cost on Mendenhall Street and Babcock Street

Treatment	Babcock Street/	Mendenhall Street/	
reaunent	Black Avenue	Tracy Ave	
High Visibility Crosswalk Markings	./	Cost \$18,000	
Thigh visibility Crosswalk Flankings	•	CRF: 40% (ped injury crashes)	
Parking Restrictions on Crosswalk Approach	Cost: \$15,500*	✓	
Tarking Restrictions on Crosswalk Approach	CRF: 30% (ped crashes)		
Adequate Nighttime Lighting Levels	Cost: \$20,000	✓	
Adequate Mighttime Lighting Levels	CRF: 42% (ped crashes)		
Crosswalk Warning Signs		Cost: \$800	
Crosswark Warning Signs	<b>Y</b>	CRF: 25% (ped crashes)	
Raised Crosswalks	Cost: Not evaluated	Cost: Not evaluated	
Naised Crosswarks	CRF: 45% (ped crashes)	CRF: 45% (ped crashes)	
In-Street Pedestrian Crossing Sign	Cost: \$1,200 each	Cost: \$1,200 each	
In-Street redestrian Crossing Sign	CRF: Unknown	CRF: Unknown	
Curb Extension	,	Cost: \$10,800	
Cui D Extension	<b>V</b>	CRF: Unknown	

<sup>\*</sup>At Babcock Street/Black Avenue, a curb extension was built to help restrict parking on the crosswalk approach.





Figure 3: Application of Pedestrian Countermeasures at Babcock Street/Black Avenue





Figure 4: Application of Pedestrian Countermeasures at Mendenhall Street/Tracy Avenue



#### CONCLUSIONS AND RECOMMENDATIONS

#### Conclusions

The preceding analysis has evaluated the safety and operations for the downtown Bozeman corridors of Babcock Street, Main Street and Mendenhall Street. An evaluation of Existing Conditions (2022) intersection capacity showed that at all intersections function at LOS C or better during the AM, noon, and PM peak hours. Pedestrian LOS was calculated to be LOS B or better at the study area intersections. A signal warrant analysis resulted in no warrants being met for the study area unsignalized intersections.

Speed data was gathered on six different segments of downtown streets and 85th percentile speeds were observed to be between 27 and 30 mph. The posted speed limit on downtown roadways is 25 mph.

A crash history analysis found that the historical crash rate was slightly higher than the predicted crash rate at the intersection of Mendenhall Street/Tracy Avenue. The historical crash rates were lower than the predicted crash rates at all other study area intersections. There were crash trends including rear-end, right-angle, sideswipe, same direction, left-turn, same direction, and pedestrian/bicycle collisions in the study area, with the most severe crashes occurring due to pedestrian and bicycle crashes and right-angle collisions.

FHWA guidance for pedestrian safety countermeasures were discussed and applied to the two example intersections of Mendenhall Street/Tracy Avenue and Babcock Street/Black Avenue. Information was provided about the crash reduction factors for the treatments and the estimated costs of application. It is suggested that these improvements be installed at other study area intersections for consistency and improved safety.

#### Recommendations

The following list of recommendations is based on the analysis results from this study and the professional judgment of the author:

At the intersection of Babcock Street/Black Avenue:

- Install high visibility pavement markings on all approaches and crosswalk warning signs and in-street pedestrian crossing signs on the eastbound approach.
- Install lighting on the northeast and southwest corners and curb extensions on the Northeast, Northwest, and Southeast corners to help improve pedestrian safety.
- Continue to evaluate intersection operations for warrants for signalization.
- Coordinate these improvements with the bike boulevard project (through the City's transportation term contract) to avoid any design conflicts.

At the intersection of Mendenhall Street/Tracy Avenue:

• Install high visibility pavement markings on all approaches and larger stop signs on Tracy Avenue to call more attention to the intersection.



- Install advanced crosswalk warning signs and in-street pedestrian crossing signs on the westbound approach to improve pedestrian safety.
- Maintain the tree on the southeast corner of the intersection to provide adequate sight distance.
- Install curb extensions on the west side of the intersection to improve pedestrian safety.

#### Throughout the downtown area:

- Provide adequately sized 15-minute loading/unloading spaces at locations where trucks frequently double park.
- Install sharrows uniformly on downtown streets.
- Restrict parking within 30' of all crosswalk approaches.
- Consolidate signage in the downtown area.
- Install lighting at intersections for pedestrian safety.
- Install high-visibility pavement marks, crosswalk warning signs, in-street pedestrian signs, curb extensions and lighting at other study area intersections for consistency and to improve safety.
- Increase the yellow clearance intervals to 3.2 seconds to match the 85th percentile speeds of 30 mph.
- Consideration should be given to continue an ongoing review of metrics.
- All transportation-related improvements shall be designed in accordance with City of Bozeman and/or MDT standards (where applicable) and the Manual on Uniform Traffic Control Devices (MUTCD).





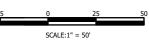
# **EXHIBIT E**

**EXISTING SIGNS** 

BABCOCK: N 5TH TO WILLSON

OCTOBER 2022

BOZEMAN, MONTANA







PREPARED FOR: CITY OF BOZEMAN

PREPARED BY:

# 398 33 BABCOCK SIGNS.dwg, MENDENHALL WILLSON TO BOZEMAN, 10/19/2022 3:28:32 PM, rboland, 1:1

# **EXHIBIT F**

EXISTING SIGNS

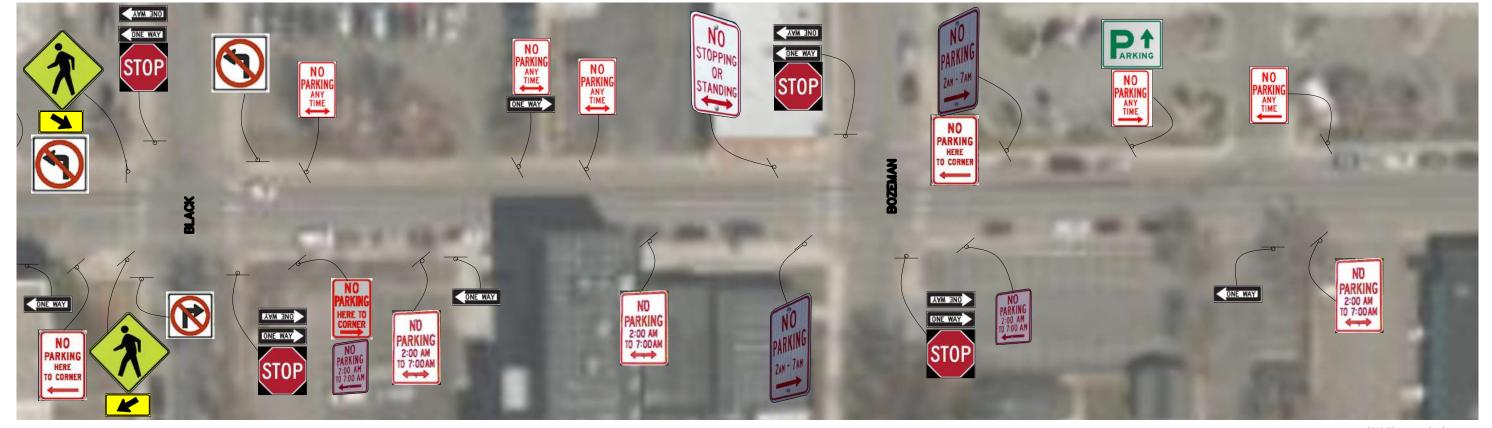
BABCOCK: WILLSON TO BOZEMAN

PREPARED FOR: CITY OF BOZEMAN

PREPARED BY : SANDERSON STEWART







# **EXHIBIT G**

**EXISTING SIGNS BABCOCK: ROUSE TO WALLACE** 



PREPARED FOR: CITY OF BOZEMAN

PREPARED BY: SANDERSON STEWART





# **EXHIBIT A**

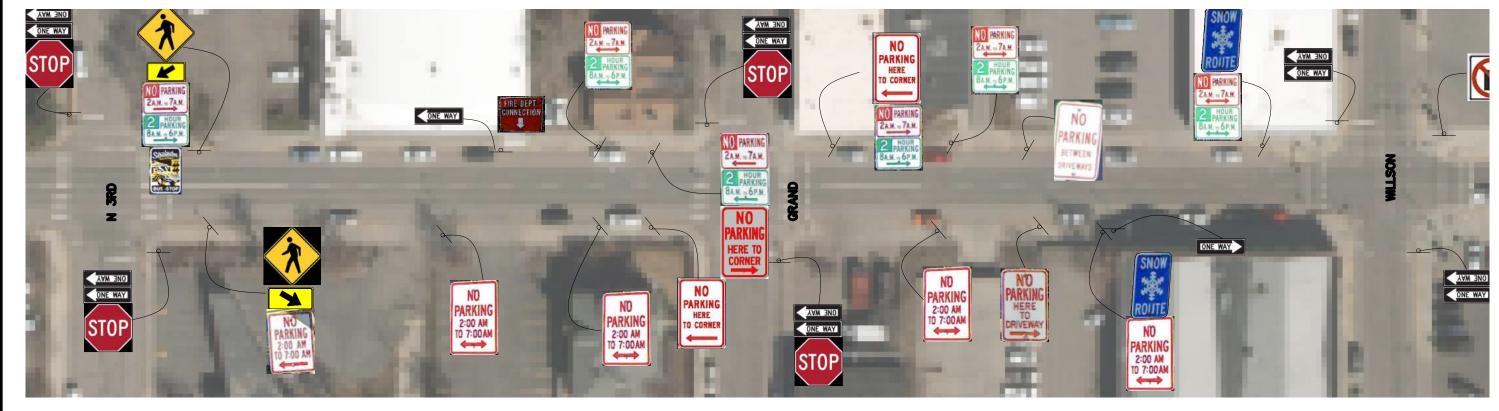
**EXISTING SIGNS** MENDENHALL: N 5TH TO WILLSON

SCALE:1" = 50'

PREPARED FOR: CITY OF BOZEMAN

PREPARED BY: SANDERSON STEWART





# **EXHIBIT B**

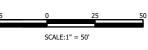
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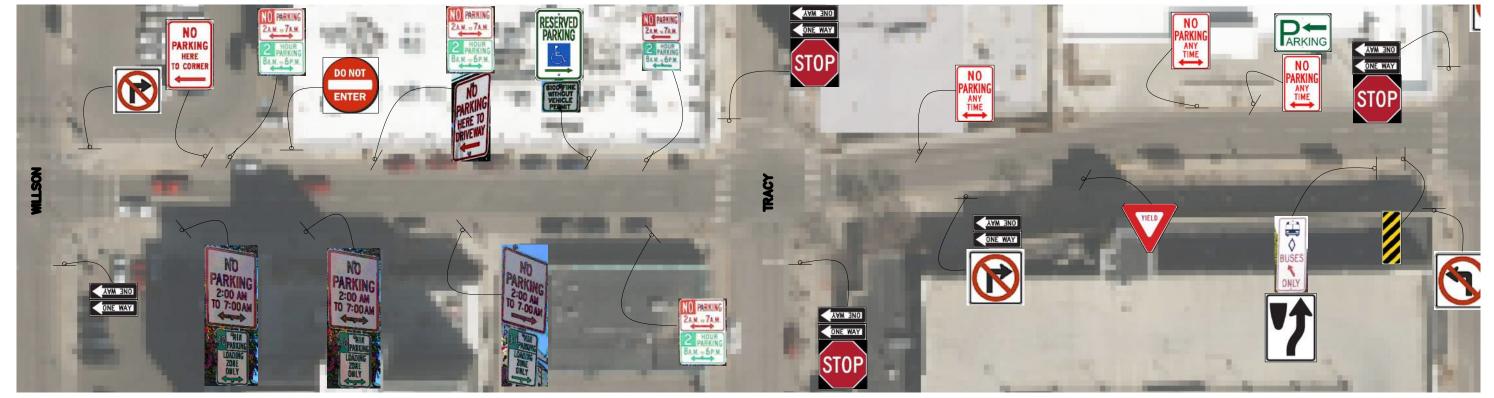
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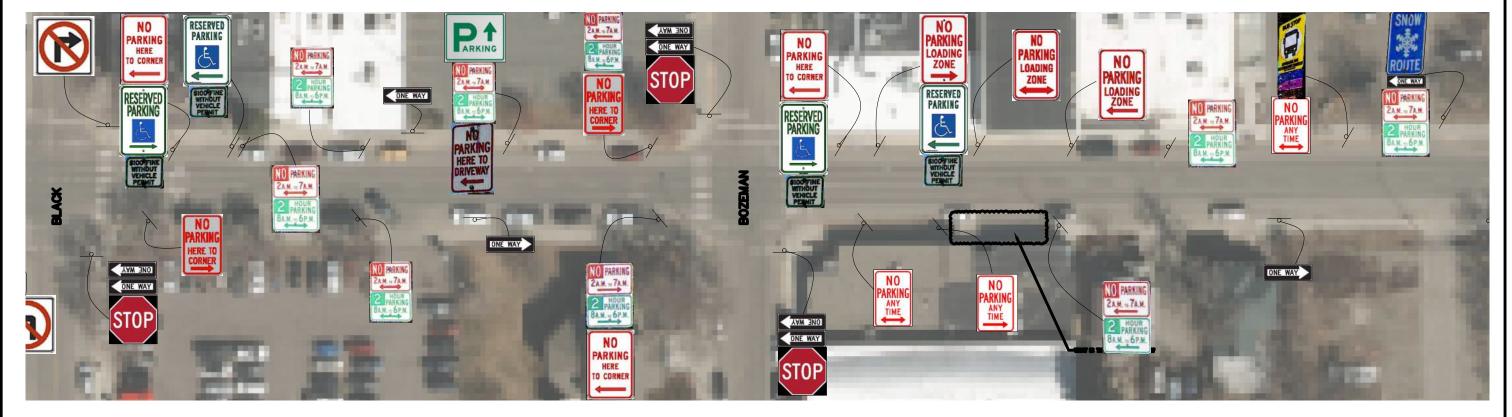


PREPARED FOR: CITY OF BOZEMAN

SANDERSON STEWART PREPARED BY:







PREPARED FOR: CITY OF BOZEMAN

# **EXHIBIT C**

**EXISTING SIGNS** 

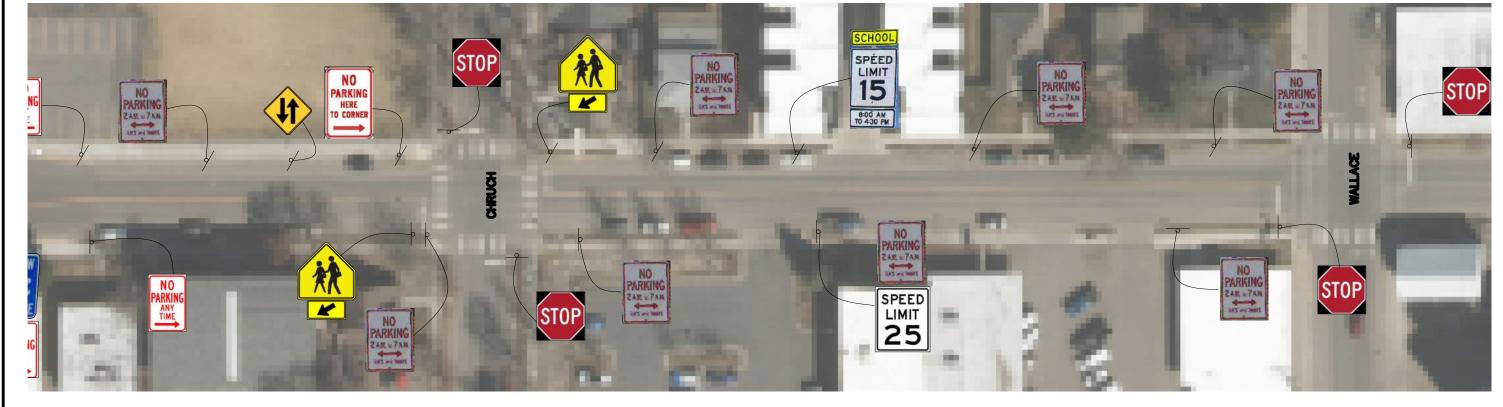
MENDENHALL: BOZEMAN TO WALLACE

OCTOBER 2022

BOZEMAN, MONTANA







# EXHIBIT D EXISTING SIGNS

MENDENHALL: WALLACE TO BROADWAY



PREPARED FOR: CITY OF BOZEMAN

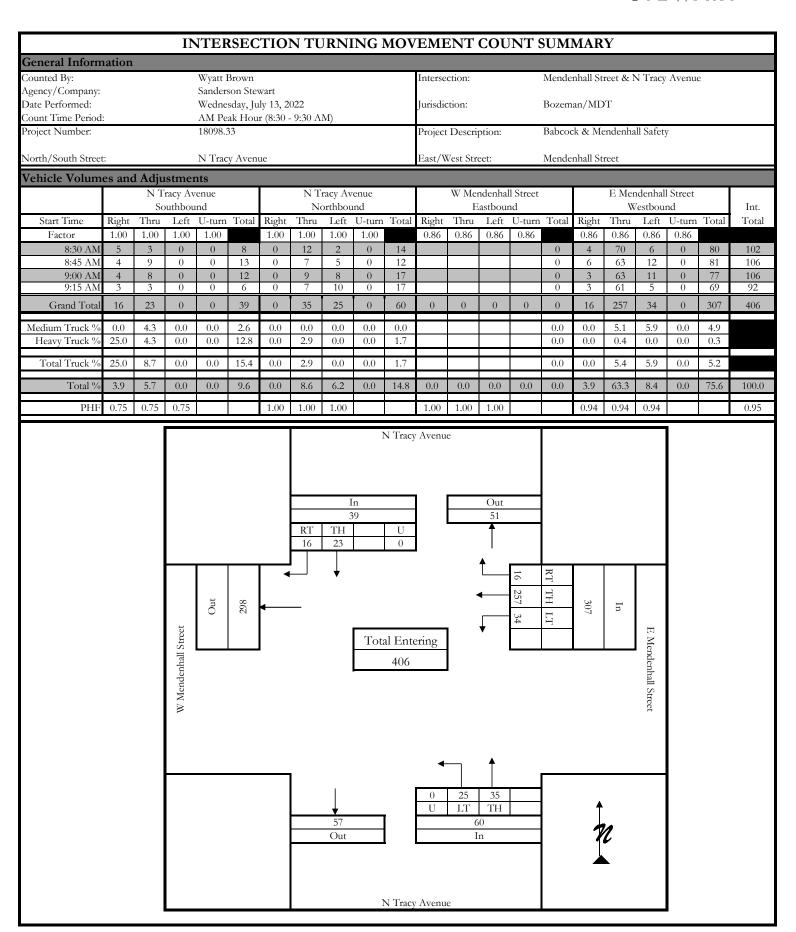
PREPARED BY : SANDERSON STEWART



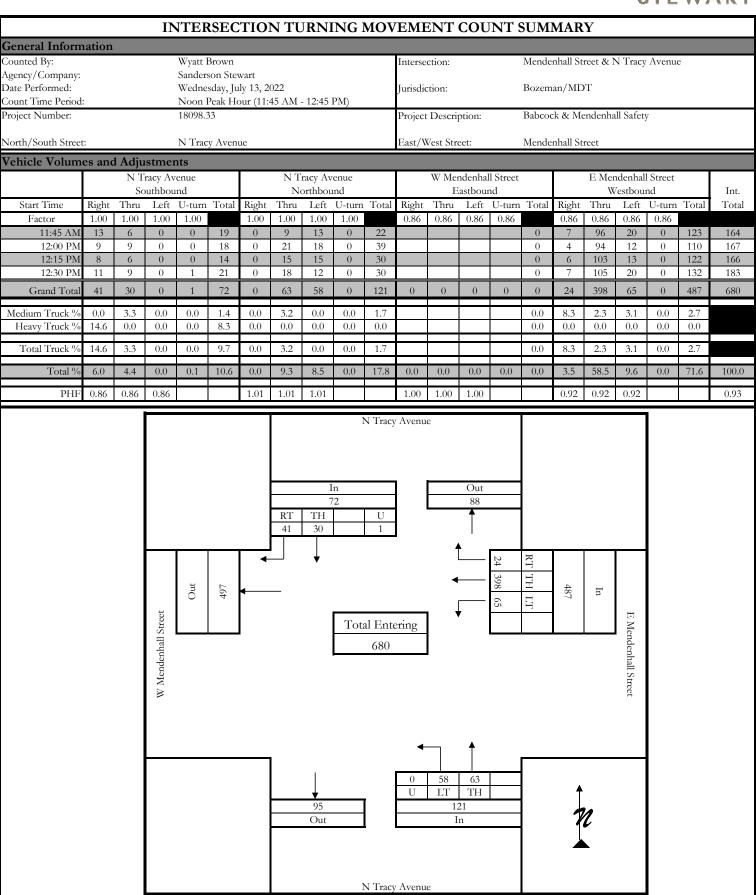




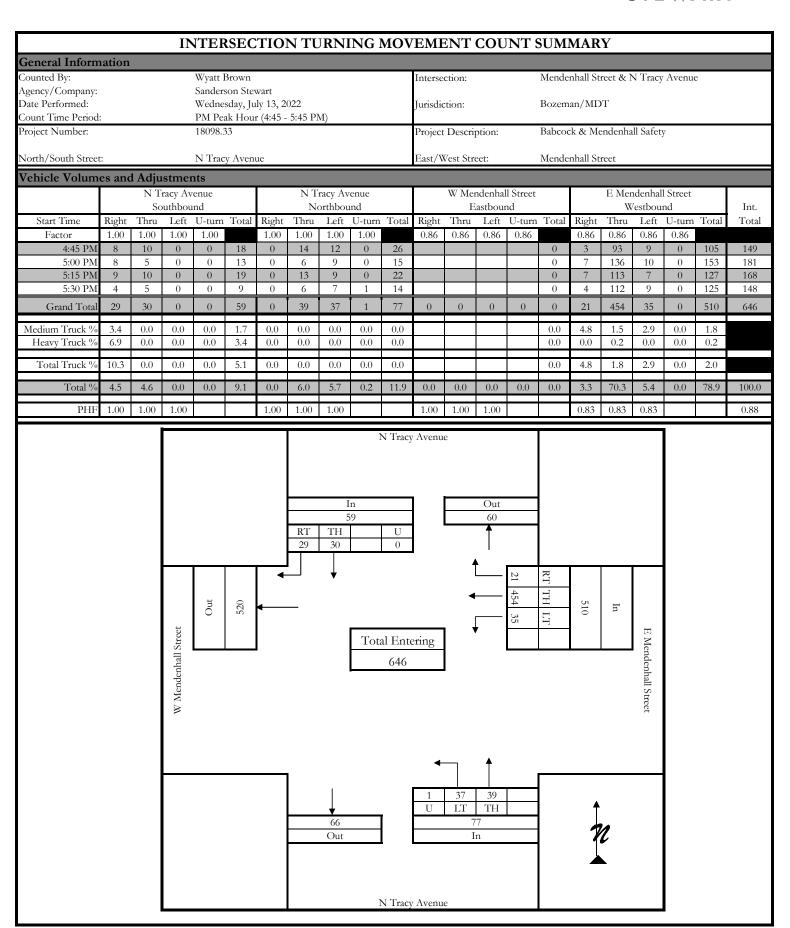














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12:00 PM   14   9   0   0   23   0   14   14   0   28   0   0   3   7   8   0   108   163   12:15 PM   18   9   0   0   27   0   14   14   0   28   0   0   0   3   77   8   0   108   163   12:20 PM   10   7   0   0   17   0   15   15   15   0   30   0   0   0   0   0   0   0	1210 PM   14   9   0   0   23   0   14   14   0   28   0   0   27   0   14   14   0   28   0   0   27   0   14   14   0   28   0   0   27   0   14   14   0   28   0   0   27   0   14   14   0   28   0   0   0   0   0   0   0   0   0						26					30	0.86	0.86	0.86	0.86	0					115	171
12.30 PM   10	12:30 PM 10	12:00 PM		9			23		14	14	0	28						6	86	17	0	109	160
Grand Total 54 39 0 0 93 0 88 58 0 116 0 0 0 0 0 0 20 380 51 0 451 660  Mediam Track 7 1.9 0.0 0.0 0.0 0.0 1.1 0.0 1.7 1.7 0.0 1.7 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	Crand Total   S4   39   0   0   93   0   88   58   0   116   0   0   0   0   0   20   380   51   0   451   666     Medium Track %   19   00   00   00   011   00   07   17   1							-															
Medium Track % 1.9 0.0 0.0 1.1 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 0.0 1.7 1.7 1.7 0.0 1.7 1.7 1.7 0.0 1.7 1.7 1.7 0.0 1.7 1.7 1.7 0.0 1.7 1.7 1.7 0.0 1.7 1.7 1.7 1.7 0.0 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	Medium Track % 19 0.0 0.0 0.0 1.1 0.0 1.7 1.7 0.0 1.7										-		-0	0			·						
Heavy Track % 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	Heavy Truck % 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0												U	0	0	U							000
Total Track % 1.9 0.0 0.0 0.0 1.1 0.0 1.7 1.7 0.0 1.7 0.0 1.7 0.0 1.7 0.0 1.7 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	Total Truck % 1.9 0.0 0.0 1.1 0.0 1.7 1.7 0.0 1.7 0.0 0.0 1.7 0.0 0.0 1.7 0.0 0.0 1.7 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0												1	<del>                                     </del>		<u> </u>							
Total N Black Avenue  N Black Avenue  N Black Avenue  N Black Avenue  Total Entering 660  Total Entering 90	Total September 10	•																					
PHF 0.89 0.89 0.89 0.97 0.97 0.97 1.00 1.00 1.00 0.98 0.98 0.98 0.98 0.96  N Black Avenue  N Black Avenue  Total Entering 660  Total Entering 660  E Mendenhall Street	PHF 0.89 0.89 0.89 0.97 0.97 0.97 1.00 1.00 1.00 0.98 0.98 0.98 0.98 0.99 0.99  N Black Avenue  N Black Avenue  Total Entering 660  Total Entering 660  Out 1.1 1.1 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5	Total Truck %	1.9	0.0	0.0	0.0	1.1	0.0	1.7	1.7	0.0	1.7					0.0	10.0			0.0	3.1	
N Black Avenue    In	N Black Avenue    In   93   78   78   78   78   78   78   78   7	Total %	8.2	5.9	0.0	0.0	14.1	0.0	8.8	8.8	0.0	17.6	0.0	0.0	0.0	0.0	0.0	3.0	57.6	7.7	0.0	68.3	100.0
Total Entering   St.	In   93   78   78   78   78   78   78   78   7	PHF	0.89	0.89	0.89			0.97	0.97	0.97			1.00	1.00	1.00			0.98	0.98	0.98			0.96
	N Black Avenue				E Mendenhall Street	Out	492	<b>*</b>		TH 39	93	0 al Ent	0	LT 1	78	380	ТН	451	In V	E Mendenhall Street			

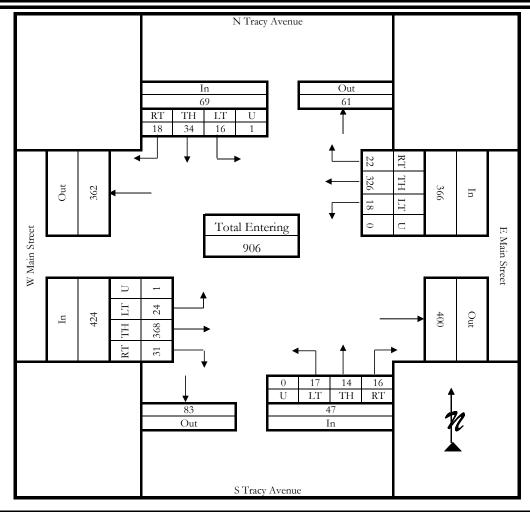


			IN	TEF	RSEC	TIO	JT V	J <b>RN</b> I	NG	MOV	EMI	ENT	COU	NT S	SUM	MAR	Y				
General Inform	nation																				
Counted By: Agency/Company:				Wyatt I	Brown son Stev	voet					Interse	ection:			E Men	denhall	Street	& N Bla	ack Ave	nue	
Date Performed:				Wedne			)22				Jurisdi	ction:			Bozem	an/MD	Т				
Count Time Period	:			PM Pe		r (4:45 -	5:45 Pl	M)			ъ :	D .			D 1	1 0 1/	1 1	11.0.0			
Project Number:				18098.3	53						Project	t Descri	ption:		Babcoo	ck & Mo	endenh	all Safet	У		
North/South Street	t:			N Blac	k Aveni	ıe					East/V	West Str	eet:		E Men	denhall	Street				
Vehicle Volum	es and																				
			lack Avouthbou					lack Av orthbou					ndenhal Eastbour					ndenha Vestbou	ll Street		Int.
Start Time	Right	Thru		U-turn	Total	Right	Thru	Left	U-turn	Total	Right	Thru		U-turn	Total	Right	Thru	Left		Total	Total
Factor	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00		0.86	0.86	0.86	0.86		0.86	0.86	0.86	0.86		
4:45 PM 5:00 PM	10	6 8	0	0	16 18	0	8 11	8	0	16 22					0	2	85 122	9 8	0	97 132	129 172
5:15 PM	8	7	0	0	15	0	10	10	0	20					0	2	110	9	0	121	156
5:30 PM	16	9	0	0	25	0	10	10	0	20					0	4	92	11	0	107	152
Grand Total	44	30	0	0	74	0	39	39	0	78	0	0	0	0	0	11	409	37	0	457	609
Medium Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0					0.0	0.0	2.2	5.4	0.0	2.4	
Heavy Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0					0.0	0.0	0.2	0.0	0.0	0.2	
Total Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0					0.0	0.0	2.4	5.4	0.0	2.6	
Total %	7.2	4.9	0.0	0.0	12.2	0.0	6.4	6.4	0.0	12.8	0.0	0.0	0.0	0.0	0.0	1.8	67.2	6.1	0.0	75.0	100.0
PHF	1.00	1.00	1.00			0.89	0.89	0.89			1.00	1.00	1.00			0.87	0.87	0.87			0.88
			E Mendenhall Street	Out	492	4	RT 44		Tot	U 0 al Ente	ering	]	Out 50	11 409 37	RT TH LT	457	În	E Mendenhall Street			
								67 Out			0 U	]	39 TH 78				e V				



	INTERSECTION TURNING	MOVEMENT COUN	T SUMMARY
General Information			
Counted By:	Wyatt Brown	Intersection:	Main Street & Tracy Avenue
Agency/Company:	Sanderson Stewart		
Date Performed:	Wednesday, July 13, 2022	Jurisdiction:	Bozeman/MDT
Count Time Period:	AM Peak Hour (8:30 - 9:30 AM)		
Project Number:	18098.33	Project Description:	Babcock & Mendenhall Safety
1 1 10 1 0	W .	F /W/ 0	M: 0
North/South Street:	Tracy Avenue	East/West Street:	Main Street

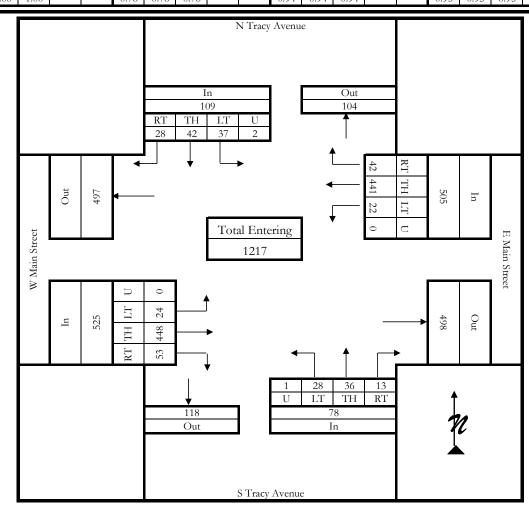
Vehicle Volum	es and	l Adju	stmei	nts																	
		N Tı	racy Av	renue			S Tı	acy Av	enue			WI	Main St	treet			Εl	Main St	reet		
		So	uthbou	ınd			No	orthbou	ınd			Е	astbou	nd			W	estbou/	nd		Int.
Start Time	Right	Thru	Left	U-turn	Total	Right	Thru	Left	U-turn	Total	Right	Thru	Left	U-turn	Total	Right	Thru	Left	U-turn	Total	Total
Factor	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00		0.85	0.85	0.85	0.85		0.85	0.85	0.85	0.85		
8:30 AM	4	7	2	1	14	3	5	3	0	11	7	92	7	0	106	7	71	6	0	84	215
8:45 AM	6	11	6	0	23	3	2	5	0	10	7	99	7	1	114	4	73	7	0	84	231
9:00 AM	4	9	5	0	18	8	3	4	0	15	7	79	6	0	92	7	85	3	0	95	220
9:15 AM	4	7	3	0	14	2	4	5	0	11	10	98	4	0	112	4	97	2	0	103	240
Grand Total	18	34	16	1	69	16	14	17	0	47	31	368	24	1	424	22	326	18	0	366	906
Medium Truck %	5.6	2.9	6.3	0.0	4.3	0.0	7.1	0.0	0.0	2.1	0.0	1.6	0.0	0.0	1.4	0.0	2.1	0.0	0.0	1.9	
Heavy Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.6	0.0	0.0	1.4	0.0	1.2	0.0	0.0	1.1	
Total Truck %	5.6	2.9	6.3	0.0	4.3	0.0	7.1	0.0	0.0	2.1	0.0	3.3	0.0	0.0	2.8	0.0	3.4	0.0	0.0	3.0	
Total %	2.0	3.8	1.8	0.1	7.6	1.8	1.5	1.9	0.0	5.2	3.4	40.6	2.6	0.1	46.8	2.4	36.0	2.0	0.0	40.4	100.0
PHF	1.00	1.00	1.00			1.00	1.00	1.00			0.94	0.94	0.94			0.89	0.89	0.89			0.94





## INTERSECTION TURNING MOVEMENT COUNT SUMMARY General Information Counted By: Wyatt Brown Main Street & Tracy Avenue Intersection: Agency/Company: Sanderson Stewart Date Performed: Wednesday, July 13, 2022 Jurisdiction: Bozeman/MDT Count Time Period: Noon Peak Hour (11:45 AM - 12:45 PM) Project Number: Project Description: Babcock & Mendenhall Safety Tracy Avenue North/South Street: East/West Street: Main Street

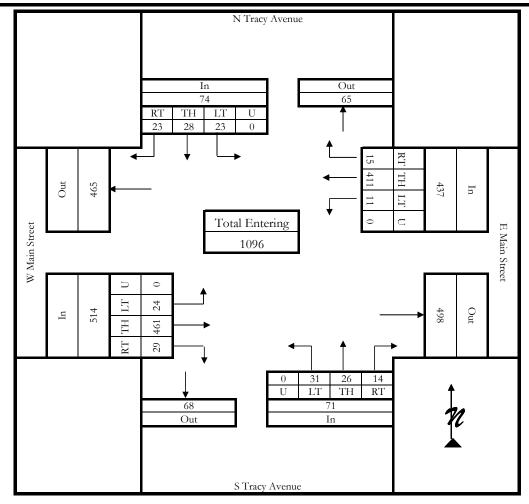
Vehicle Volum	es and	l Adju	stme	nts																	
		N T	racy Av	enue			S Tı	acy Av	enue			W	Main St	reet			Εl	Main St	reet		
		So	uthbou	ınd			No	orthbou	ınd			Е	astbou	nd			W	/estbou	nd		Int.
Start Time	Right	Thru	Left	U-turn	Total	Right	Thru	Left	U-turn	Total	Right	Thru	Left	U-turn	Total	Right	Thru	Left	U-turn	Total	Total
Factor	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00		0.85	0.85	0.85	0.85		0.85	0.85	0.85	0.85		
11:45 AM	6	12	14	1	33	4	7	6	1	18	13	103	3	0	119	14	119	5	0	138	308
12:00 PM	7	13	1	1	22	4	9	8	0	21	17	99	9	0	125	12	90	5	0	107	275
12:15 PM	7	7	8	0	22	3	12	10	0	25	9	127	4	0	140	8	118	6	0	132	319
12:30 PM	8	10	14	0	32	2	8	4	0	14	14	119	8	0	141	8	114	6	0	128	315
Grand Total	28	42	37	2	109	13	36	28	1	78	53	448	24	0	525	42	441	22	0	505	1217
Medium Truck %	0.0	0.0	5.4	0.0	1.8	0.0	2.8	3.6	0.0	2.6	0.0	1.1	0.0	0.0	1.0	4.8	2.5	0.0	0.0	2.6	
Heavy Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.6	4.2	0.0	1.5	0.0	1.6	0.0	0.0	1.4	
Total Truck %	0.0	0.0	5.4	0.0	1.8	0.0	2.8	3.6	0.0	2.6	0.0	2.7	4.2	0.0	2.5	4.8	4.1	0.0	0.0	4.0	
Total %	2.3	3.5	3.0	0.2	9.0	1.1	3.0	2.3	0.1	6.4	4.4	36.8	2.0	0.0	43.1	3.5	36.2	1.8	0.0	41.5	100.0
PHF	1.00	1.00	1.00			0.78	0.78	0.78			0.94	0.94	0.94			0.95	0.95	0.95			0.95





	INTERSECTION TURNING	S MOVEMENT COUN	1 SUMMARY
General Information			
Counted By:	Wyatt Brown	Intersection:	Main Street & Tracy Avenue
Agency/Company:	Sanderson Stewart		
Date Performed:	Wednesday, July 13, 2022	Jurisdiction:	Bozeman/MDT
Count Time Period:	PM Peak Hour (4:45 - 5:45 PM)		
Project Number:	18098.33	Project Description:	Babcock & Mendenhall Safety
North/South Street:	Tracy Avenue	East/West Street:	Main Street

Vehicle Volum	es and	l Adju	stme	nts																	
		N T	racy Av	renue			S Tı	acy Av	enue			W	Main St	treet			ΕN	Main St	reet		
		So	uthbou	ınd			No	orthbou	ınd			Е	astbou					estbou	nd		Int.
Start Time	Right	Thru	Left	U-turn	Total	Right	Thru	Left	U-turn	Total	Right	Thru	Left	U-turn	Total	Right	Thru	Left	U-turn	Total	Total
Factor	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00		0.85	0.85	0.85	0.85		0.85	0.85	0.85	0.85		
4:45 PM	7	9	5	0	21	3	8	8	0	19	8	125	9	0	142	4	92	1	0	97	279
5:00 PM	4	8	7	0	19	4	5	12	0	21	11	104	4	0	119	5	111	6	0	122	281
5:15 PM	7	4	6	0	17	5	8	5	0	18	3	117	8	0	128	3	106	3	0	112	275
5:30 PM	5	7	5	0	17	2	5	6	0	13	7	115	3	0	125	3	102	1	0	106	261
Grand Total	23	28	23	0	74	14	26	31	0	71	29	461	24	0	514	15	411	11	0	437	1096
Medium Truck %		0.0	4.3	0.0	1.4	14.3	0.0	0.0	0.0	2.8	0.0	0.7	0.0	0.0	0.6	0.0	0.2	0.0	0.0	0.2	
Heavy Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.1	0.0	0.0	1.0	0.0	0.5	0.0	0.0	0.5	
Total Truck %	0.0	0.0	4.3	0.0	1.4	14.3	0.0	0.0	0.0	2.8	0.0	1.7	0.0	0.0	1.6	0.0	0.7	0.0	0.0	0.7	
Total %	2.1	2.6	2.1	0.0	6.8	1.3	2.4	2.8	0.0	6.5	2.6	42.1	2.2	0.0	46.9	1.4	37.5	1.0	0.0	39.9	100.0
PHF	0.97	0.97	0.97			0.85	0.85	0.85			1.00	1.00	1.00			0.90	0.90	0.90			0.98





			IN	<b>ITE</b>	RSEC	TIO	N TU	RNI	NG :	MOV	EMI	ENT	COU	NT	SUM	MAR	Y				
General Inform	ation																				j
Counted By:				Wyatt 1							Interse	ction:			E Main	Street	& Black	. Avenu	ıe		
Agency/Company: Date Performed:					son Stev sday, Ju		022				Jurisdio	rtion:			Bozem	an/MD	т				
Count Time Period:	:				ak Hou			M)			Janoan	Juon.			DOZEIII	an, 1112	1				
Project Number:				18098.3		•		,			Project	Descri	ption:		Babcoo	ck & Me	endenha	ıll Safet	у		
North/South Street				Black A	Venue						East/V	Vest Str	eet.		E Main	Street					
Vehicle Volume		1 Adin	otm or		rverrae						Base/ v	vest ou	ect.		E Main	rotrect					
venicie volum	es and		lack Av				S BI	ack Av	enne			E.I	Main St	reet			E.I	Main St	reet		
			outhbou					orthbou					astbour					estbou			Int.
Start Time	Right	Thru		U-turn	Total	Right	Thru		U-turn	Total	Right	Thru		U-turn	Total	Right	Thru		U-turn	Total	Total
Factor	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00		0.85	0.85	0.85	0.85	0.5	0.85	0.85	0.85	0.85		
8:30 AM 8:45 AM	4	8 9	4	0	16 17	4	4	4	0	12 12	5 4	92 92	9	0	95 106	5	83 79	3	0	92 87	215 222
9:00 AM	5	9	5	0	19	5	5	5	0	15	13	81	9	0	106	6	94	3	0	103	240
9:15 AM	5	10	5	0	20	5	5	5	0	15	7	96	11	0	114	7	106	4	0	117	266
Grand Total	18	36	18	0	72	18	18	18	0	54	29	351	37	1	418	23	362	14	0	399	943
Medium Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.7	0.0	0.0	1.4	0.0	1.4	0.0	0.0	1.3	
Heavy Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.7	0.0	0.0	1.4	0.0	1.1	0.0	0.0	1.0	
Total Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.4	0.0	0.0	2.9	0.0	2.5	0.0	0.0	2.3	
Total %	1.9	3.8	1.9	0.0	7.6	1.9	1.9	1.9	0.0	5.7	3.1	37.2	3.9	0.1	44.3	2.4	38.4	1.5	0.0	42.3	100.0
PHF	0.90	0.90	0.90			0.90	0.90	0.90			0.92	0.92	0.92			0.85	0.85	0.85			0.89
			E Main Street	Out	399	<b>*</b>	RT 18	TH 36	Tot	U 0  ■ al Ent	ering	•	† 	23 362 14 0	RT TH LT U	399	In	E Main Street			
			E Mai	In	418	RT TH LT U	29 351 37 1		<b>↑</b>		•		<b>↑</b>	_	<b></b>	387	Out	Street			
								79 Out		]	0 U		18 TH 4	18 RT							



General Inform			11	1111											• • • • • • • • • • • • • • • • • • • •						
	nation					110	111	71(1)	110	VIO V	LIVII	21 1 1	COC	NT S	OWI	VIII	. 1				
Counted By:	11411011			Wyatt 1	Brown						Interse	ction:			E Mair	Street	& Black	Avenu	ıe		
Agency/Company				Sander			^~~								D	/a.m					
Date Performed: Count Time Period	1.			Wedne			022 45 AM -	12.45	PM)		Jurisdio	ction:			Bozem	an/MD	0.1.				
Project Number:				18098.3		7d1 (11.	15 7 1111	12.13	1 1,1)		Project	Descri	ption:		Babcoo	ck & Me	endenha	ll Safet	y		
NI 4 /0 4 0				D1 1 /							F . /V	77 . C.			EM:	C					
North/South Stree		1 4 11		Black /	lvenue						East/V	Vest Str	eet:		E Mair	Street					
Vehicle Volum	nes and		l <b>stme</b> r lack Av				C DI	ack Av	0.0017.0		1	17.1	Main St	#oot			E 7	Main St	#oot		
			outhbou					ack Ave					astbou					viani st Vestbou			Int.
Start Time	Right	Thru	Left	U-turn	Total	Right	Thru		U-turn	Total	Right	Thru	Left	U-turn	Total	Right	Thru		U-turn	Total	Total
Factor	1.00	1.00	1.00	1.00	2.2	1.00	1.00	1.00	1.00	45	0.85	0.85	0.85	0.85	101	0.85	0.85	0.85	0.85	106	207
11:45 AM 12:00 PM		8	8	0	22	3	8	6 5	0	17 16	6 7	103 90	12 11	0	121 108	7	126 114	7	0	136 128	296 273
12:15 PM		10	10	0	26	3	10	6	0	19	3	133	14	0	150	8	131	3	0	142	337
12:30 PM	6	9	9	0	24	3	9	6	0	18	8	119	13	0	140	8	123	3	0	134	316
Grand Total	23	35	35	0	93	12	35	23	0	70	24	445	50	0	519	30	494	16	0	540	1222
Medium Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.3	0.0	0.0	1.2	0.0	2.0	0.0	0.0	1.9	
Heavy Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.6	0.0	0.0	1.3	0.0	1.0	0.0	0.0	0.9	
Total Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.9	0.0	0.0	2.5	0.0	3.0	0.0	0.0	2.8	
Total %	1.9	2.9	2.9	0.0	7.6	1.0	2.9	1.9	0.0	5.7	2.0	36.4	4.1	0.0	42.5	2.5	40.4	1.3	0.0	44.2	100.0
PHF		0.89	0.89	0.0	7.0	0.92	0.92	0.92	0.0	3.7	0.87	0.87	0.87	0.0	12.0	0.95	0.95	0.95	0.0	1112	0.91
PHF	0.89	0.89	0.89			0.92	0.92	0.92			0.87	0.87	0.87			0.95	0.95	0.95			0.91
			E Main Street	Out	540	<b>+</b>	RT 23		Tot	U 0	ering	,	Out 115	30 494 16 0	RT TH LT U	540	In	E Main Street			
			EN	In	519	RT TH LT U	24 445 50 0	75 Out	<b>→</b>		0 U		35 TH	12 RT	<u> </u>	492	Out	eet			

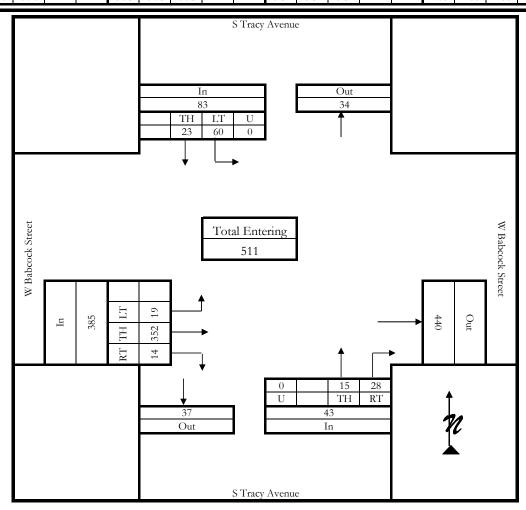


			IN	TEF	RSEC	TIO	N TU	J <b>RN</b> I	NG :	MOV	EMI	ENT	COU	NT S	SUM	MAR	Y				
General Inform	ation																				
Counted By:				Wyatt 1							Interse	ection:			E Mair	Street	& Black	. Avenu	ıe		
Agency/Company: Date Performed:				Sander		wart ly 13, 20	022				Iurisdi	ation.			Rozom	an/MD	т				
Count Time Period:						r (4:45 -		M)			Jurisai	cuon:			Dozem	a11/ MIL	<b>,</b> 1				
Project Number:				18098.		(		/			Projec	t Descri	ption:		Babcoo	ck & Mo	endenha	ıll Safet	y		
North/South Street				Black A	Venue						East/V	West Str	eet.		E Mair	St <del>r</del> eet					
Vehicle Volume		l Adiu	ıstmer		ivenue						East)	vest sti	eet.		13 IVIAN	Burect					
, cinete , otorii	00 4414		lack Av				S Bl	lack Av	enue			E	Main St	reet			Εl	Main St	reet		
			outhbou					orthbou					Eastbou					estbou			Int.
Start Time	Right	Thru		U-turn	Total		Thru	Left	U-turn	Total	Right		Left	U-turn	Total	Right	Thru	Left	U-turn	Total	Total
Factor 4:45 PM	1.00	1.00	1.00	1.00	17	1.00	1.00	1.00	1.00	18	0.85	0.85	0.85	0.85	129	0.85	0.85 95	0.85	0.85	101	265
5:00 PM	5	3	5	0	13	3	6	9	0	18	11	86	10	0	107	4	123	3	0	130	268
5:15 PM	4	5	7	0	16	3	6	9	0	18	6	112	12	0	130	1	106	3	0	110	274
5:30 PM	9	10	10	0	29	3	6	9	0	18	8	95	13	0	116	6	106	2	0	114	277
Grand Total	23	24	28	0	75	12	24	36	0	72	33	403	46	0	482	14	430	11	0	455	1084
Medium Truck %	0.0	0.0	7.1	0.0	2.7	0.0	0.0	0.0	0.0	0.0	0.0	1.7	0.0	0.0	1.5	0.0	0.0	0.0	0.0	0.0	
Heavy Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.5	0.0	0.0	1.2	0.0	0.5	0.0	0.0	0.4	
Total Truck %	0.0	0.0	7.1	0.0	2.7	0.0	0.0	0.0	0.0	0.0	0.0	3.2	0.0	0.0	2.7	0.0	0.5	0.0	0.0	0.4	
Total %	2.1	2.2	2.6	0.0	6.9	1.1	2.2	3.3	0.0	6.6	3.0	37.2	4.2	0.0	44.5	1.3	39.7	1.0	0.0	42.0	100.0
PHF	0.65	0.65	0.65			1.00	1.00	1.00			1.00	1.00	1.00			1.00	1.00	1.00			0.98
											x Avenu								1		
				Out	489	<b>+</b>	RT 23	TH 24	75 LT 28	U 0		, ,	84	14 430 11	RT TH LT U	455	In				
			E Main Street	In	482	RT TH LT U	33 403 46 0	68 Out	Tot	al Ent 1084			1 24 TH 72 In	12 RT	<u> </u>	443	Out	E Main Street			
										S Black	. Avenu	e									



	INTERSECTION TURNING	MOVEMENT COUN	T SUMMARY
General Information			
Counted By:	Wyatt Brown	Intersection:	W Babcock Street & S Tracy Avenue
Agency/Company:	Sanderson Stewart		
Date Performed:	Wednesday, July 13, 2022	Jurisdiction:	Bozeman/MDT
Count Time Period:	AM Peak Hour (8:30 - 9:30 AM)		
Project Number:	18098.33	Project Description:	Babcock & Mendenhall Safety
North/South Street:	S Tracy Avenue	East/West Street:	W Babcock Street

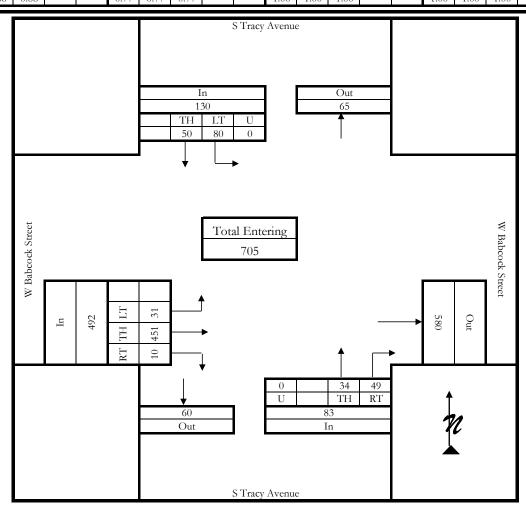
Vehicle Volum	es and	l Adju	stmei	nts																	
		S Tr	acy Av	enue			S Tı	acy Av	enue			W Ba	bcock	Street			W Ba	abcock	Street		
		So	uthbou	nd			No	orthbou	ınd			Е	astbou	nd			W	estbou/	nd		Int.
Start Time	Right	Thru	Left	U-turn	Total	Right	Thru	Left	U-turn	Total	Right	Thru	Left	U-turn	Total	Right	Thru	Left	U-turn	Total	Total
Factor	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00		0.86	0.86	0.86	0.86		0.86	0.86	0.86	0.86		
8:30 AM	0	7	11	0	18	5	3	0	0	8	3	86	4	0	93					0	119
8:45 AM	0	6	17	0	23	9	2	0	0	11	3	112	4	0	119					0	153
9:00 AM	_	7	15	0	22	8	5	0	0	13	4	82	4	0	90					0	125
9:15 AM	0	3	17	0	20	6	5	0	0	11	4	72	7	0	83					0	114
Grand Total	0	23	60	0	83	28	15	0	0	43	14	352	19	0	385	0	0	0	0	0	511
Medium Truck %	0.0	0.0	1.7	0.0	1.2	0.0	0.0	0.0	0.0	0.0	0.0	1.7	0.0	0.0	1.6					0.0	
Heavy Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.3	0.0	0.0	0.3					0.0	
Total Truck %	0.0	0.0	1.7	0.0	1.2	0.0	0.0	0.0	0.0	0.0	0.0	2.0	0.0	0.0	1.8					0.0	
Total %	0.0	4.5	11.7	0.0	16.2	5.5	2.9	0.0	0.0	8.4	2.7	68.9	3.7	0.0	75.3	0.0	0.0	0.0	0.0	0.0	100.0
PHF	0.90	0.90	0.90			0.98	0.98	0.98			0.81	0.81	0.81			1.00	1.00	1.00			0.83





## INTERSECTION TURNING MOVEMENT COUNT SUMMARY **General Information** Wyatt Brown W Babcock Street & S Tracy Avenue Counted By: Intersection: Agency/Company: Sanderson Stewart Date Performed: Wednesday, July 13, 2022 Jurisdiction: Bozeman/MDT Count Time Period: Noon Peak Hour (11:45 AM - 12:45 PM) Project Number: Project Description: Babcock & Mendenhall Safety S Tracy Avenue W Babcock Street North/South Street: East/West Street:

Vehicle Volum	es and	l Adju	stmei	nts																	
		S Tr	acy Av	enue			S Tı	acy Av	enue			W Ba	bcock	Street			W Ba	abcock	Street		
		So	uthbou	ınd			No	orthbou	ınd			Е	astbou	nd			W	estbou/	nd		Int.
Start Time	Right	Thru	Left	U-turn	Total	Right	Thru	Left	U-turn	Total	Right	Thru	Left	U-turn	Total	Right	Thru	Left	U-turn	Total	Total
Factor	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00		0.86	0.86	0.86	0.86		0.86	0.86	0.86	0.86		
11:45 AM	0	10	21	0	31	10	8	0	0	18	3	102	6	0	111					0	160
12:00 PM		15	21	0	36	10	8	0	0	18	0	116	8	0	124					0	178
12:15 PM	0	13	13	0	26	11	9	0	0	20	3	118	14	0	135					0	181
12:30 PM	0	12	25	0	37	18	9	0	0	27	4	115	3	0	122					0	186
Grand Total	0	50	80	0	130	49	34	0	0	83	10	451	31	0	492	0	0	0	0	0	705
Medium Truck %	0.0	0.0	0.0	0.0	0.0	0.0	2.9	0.0	0.0	1.2	0.0	1.3	3.2	0.0	1.4					0.0	
Heavy Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0					0.0	
Total Truck %	0.0	0.0	0.0	0.0	0.0	0.0	2.9	0.0	0.0	1.2	0.0	1.3	3.2	0.0	1.4					0.0	
Total %	0.0	7.1	11.3	0.0	18.4	7.0	4.8	0.0	0.0	11.8	1.4	64.0	4.4	0.0	69.8	0.0	0.0	0.0	0.0	0.0	100.0
PHF	0.88	0.88	0.88			0.77	0.77	0.77			1.00	1.00	1.00			1.00	1.00	1.00			0.95



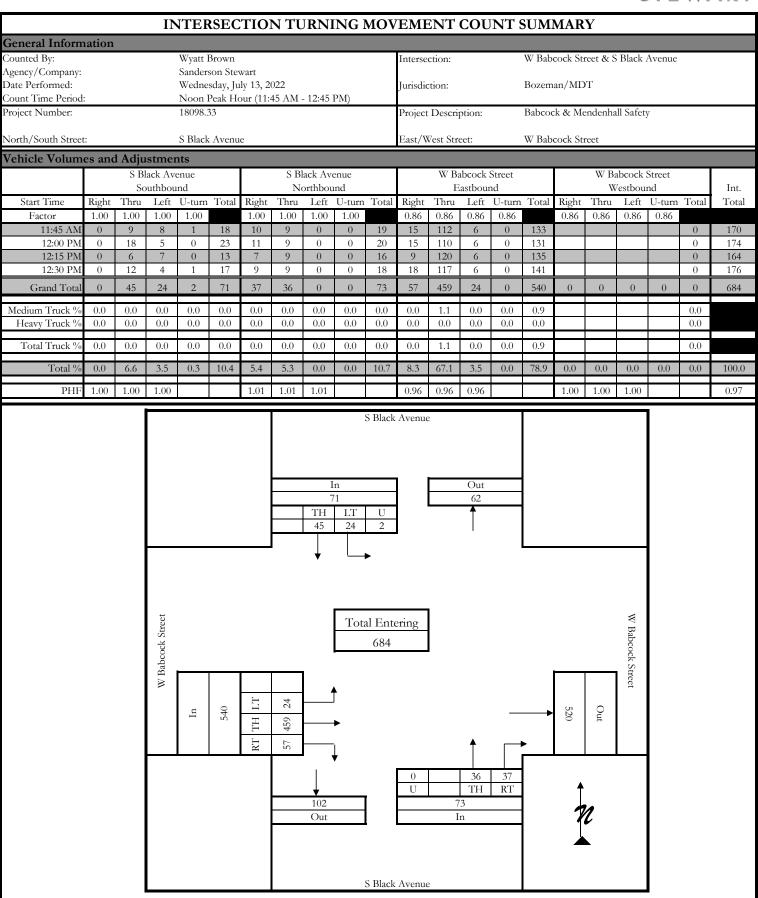


Side   PM   O	Courted By: Sanderson Stewart Date Performed: Orant Time Repict Time Right Time Left U-tum Total Right Time Left U-tum Total Right Time Left U-tum Total Time Factor 100 100 100 100 100 100 100 100 100 10				IN	TER	SEC	TIO	N TU	RNI	NG	MOV	EMI	ENT	COU	NT:	SUM	MAR	RY				
Sundame	Magney Compunes   Sunderson Stewart   Magney Compunes   Wednesday 1913, 202   Durisdictions   Bozeman/MIDT		nation	ı																			
Date Performed:	Date Performed:												Interse	ection:			W Bab	cock St	reet & S	S Tracy	Avenue	;	
Count Time Period:   PM Peak Hour (4:45 - 5:45 PM)   Project Description:   Babcock & Mendenhall Safety	Count Time   Period:   PM   Peak   Hour (445 - 545   PM)								022				Inriedi	ction:			Bozem	an/MF	YΤ				
North/South Street   S Tracy Avenue	North/South Street   S Tracy Avenue		:							M)			Jurisci	cuon.			DOZEIII	an ivit	, 1				
S   Tracy Avenue	STracy Avenue	Project Number:				18098.	33	•		•			Projec	t Descri	ption:		Babcoo	ck & Mo	endenh	all Safet	ty		
Vehicle   Volumes and Adjustments	Vehicle Volumes and Adjustments	North/South Street	t:			S Trac	y Avenu	ie					East/V	West Str	eet:		W Bab	cock St	reet				
Sourt Time   Right   The Left   Usure   Total   Total   Usure   Total   Total   Right   The Left   Usure   Total   Right   The Left   Usure   Total   Right   Total   Usure   Total   Total	SourtTime   Right   Thru   Celt   U-turn   Total   U-turn   Total   Celt   U-turn   Total   Celt   U-turn   Total   U-turn   Total   U-turn   Total   Celt   U-turn   Total   U-tu	Vehicle Volume	es and	d Adju	ıstmeı	nts																	
Start Time	Start Time																						-
Fetor	Factor 100 1.00 1.00 1.00 1.00 1.00 1.00 1.00	Start Time	Right				Total	Right				Total	Right				Total	Right				Total	
## 345 PM 0 9 5 0 14 4 6 0 0 0 10 4 8 80 12 0 0 6 6 0 0 12	## 445 PM 0 9 5 5 0 14 4 4 6 0 0 0 10 4 80 12 0 96						Total					Total					Total					Total	10ta
\$15 PM 0 7 7 3 0 10 6 9 0 0 15 4 977 5 0 106	St3 PM   0		0				14					10	_				96					0	120
Sign   Mode   Sign   Find   Sign	Signat   Out   Signature   S			_																		_	154
Grand Total 0 35 37 0 72 22 31 0 0 53 14 365 25 0 404 0 0 0 0 0 0 0 5 5 5 5 5 5 5 5 5	Grand Total 0 35 37 0 72 22 31 0 0 0 53 14 365 25 0 404 0 0 0 0 0 0 0 0 52  Medium Truck % 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0									_												-	
Medium Truck % 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	Medium Truck % 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0											•				-		0	0	0		-	
Heavy Truck % 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	Heavy Truck %   0.0																	U	0	0	0		529
Total Truck % 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	Total Track % 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0											_											
Total % 0.0 6.6 7.0 0.0 13.6 4.2 5.9 0.0 0.0 10.0 2.6 69.0 4.7 0.0 76.4 0.0 0.0 0.0 0.0 0.0 0.0 0.0 100.4 PHF 0.58 0.58 0.58 1.00 1.00 1.00 1.00 1.00 0.90 0.90 0.90	Total % 0.0 6.6 7.0 0.0 13.6 4.2 5.9 0.0 0.0 10.0 2.6 69.0 4.7 0.0 76.4 0.0 0.0 0.0 0.0 0.0 100.4 PHF 0.58 0.58 0.58 0.58 0.58 0.58 0.58 0.58		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	4.0	0.0	0.2					0.0	
PHF 0.58 0.58 0.58	PHIF 0.58 0.58 0.58	Total Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.4	4.0	0.0	1.5					0.0	
Total Entering  529  W Baboods Street  Total Entering  529	In   Out   72   56   TH   LT   U   35   37   0     Out   56     Out   Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56	Total %	0.0	6.6	7.0	0.0	13.6	4.2	5.9	0.0	0.0	10.0	2.6	69.0	4.7	0.0	76.4	0.0	0.0	0.0	0.0	0.0	100.0
Total Entering   S Tracy Avenue   S Tr	In   Out   72   56   The LT   U   35   37   0     Out   56     Out   Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56     Out   56   O	PHE	0.58	0.58	0.58			1.00	1.00	1.00			0.90	0.90	0.90			1.00	1.00	1.00			0.86
In Out 56  TH LT U 56  Total Entering 529  Total Entering 529	In	1111	0.00	0.00	0.50			1.00	1.00	1.00			0.70	0.70	0.70			1100	1.00	1100			0.00
	Out 424				cock Street					TH	72   LT   37	0 ▶	ering							W Babcod			
					W Bal	In	404			4	<u>†</u>						<b></b>	424	Out	Street			
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U TH RT ↑												}	_		TH 3			1	Z.				

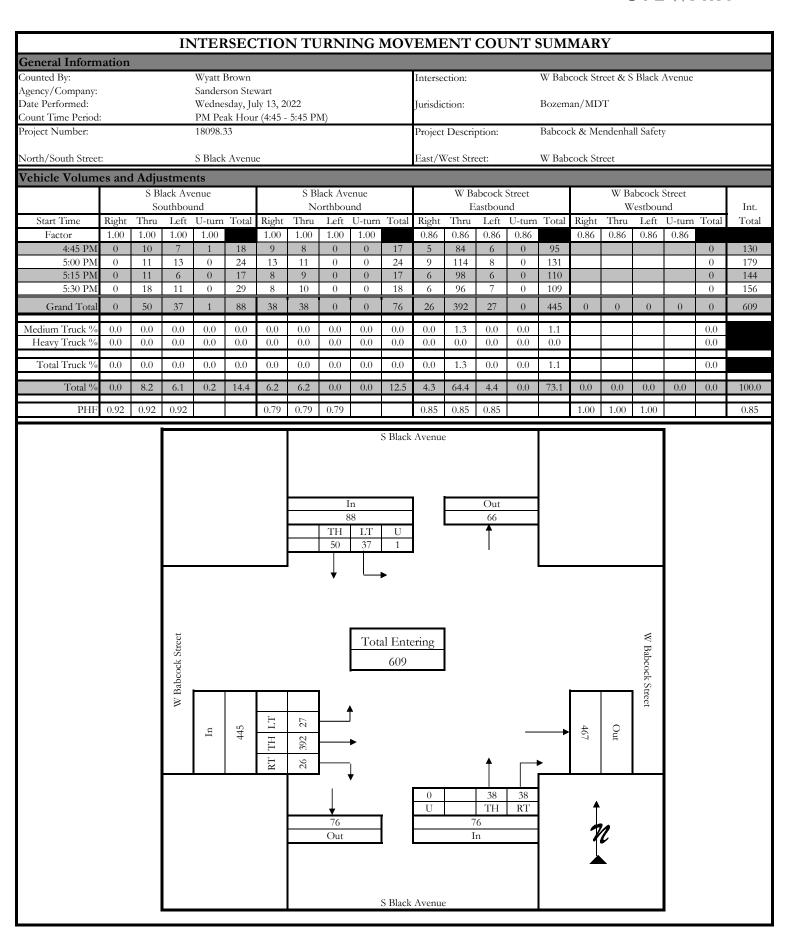


			IN	TEF	RSEC	TIO	N TU	J <b>RN</b> I	NG :	MOV	EMI	ENT	COU	NT S	SUM	MAR	Y				
General Inform	ation																				
Counted By:				Wyatt 1							Interse	ction:			W Bab	cock Stı	reet & S	Black .	Avenue		
Agency/Company: Date Performed:				Sander Wedne		wart ly 13, 20	022				Jurisdio	ction.			Bozem	an/MD	Т				
Count Time Period:	:						- 9:30 A	M)			Jansan	ouon.			Dozem	an, 1111	. 1				
Project Number:				18098.3	33						Project	Descri	ption:		Babcoc	k & Me	endenha	all Safet	У		
North/South Street	:			S Black	Avenu	e					East/V	Vest Str	eet:		W Bab	cock Stı	reet				
Vehicle Volume	es and	l Adju	stmer	nts																	
		S Bl	ack Ave	enue				lack Av					abcock					abcock			
Start Time	Diolet	So	uthbou Left	nd U-turn	Total	Diolet	No Thru	orthbou Left	ınd U-turn	Total	Right	Thru	astbou Left	nd U-turn	Total	Diolet	Thru	estbou Left	nd U-turn	Total	Int. Total
Factor	Right 1.00	1.00	1.00	1.00	Total	1.00	1.00	1.00	1.00	Total	0.86	0.86	0.86	0.86	Total	Right 0.86	0.86	0.86	0.86	Total	Total
8:30 AM	0	8	4	0	12	5	4	0	0	9	6	92	4	0	102	0.00	0.00	0.00	0.00	0	123
8:45 AM	0	11	4	0	15	5	5	0	0	10	9	112	6	0	127					0	152
9:00 AM 9:15 AM	0	7 8	3	0	11 12	15 11	3	0	0	19 14	10 5	77 73	4	0	91 82					0	121 108
Grand Total		34	15	1	50	36	16	0	0	52	30	354	18	0	402	0	0	0	0	0	504
Medium Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.1	0.0	0.0	1.0					0.0	
Heavy Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.3	0.0	0.0	0.2					0.0	
Total Truck %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.4	0.0	0.0	1.2					0.0	
Total %	0.0	6.7	3.0	0.2	9.9	7.1	3.2	0.0	0.0	10.3	6.0	70.2	3.6	0.0	79.8	0.0	0.0	0.0	0.0	0.0	100.0
PHF	0.83	0.83	0.83			1.00	1.00	1.00			0.80	0.80	0.80			1.00	1.00	1.00			0.83
											Avenue										
									LT 15	U 1			Out 35								
			eet					<b>\</b>	Tot	<b>▶</b> al Ent	erino	]		•				W			
			W Babcock Street					I		504					Ī			W Babcock Street			
			W	In	402	RT TH LT	30 354 18		<b>↑</b> <b>→</b>				<b>↑</b>	_	<b></b>	405	Out	et			
								64 Out			0 U		16 TH 2	36 RT			l				

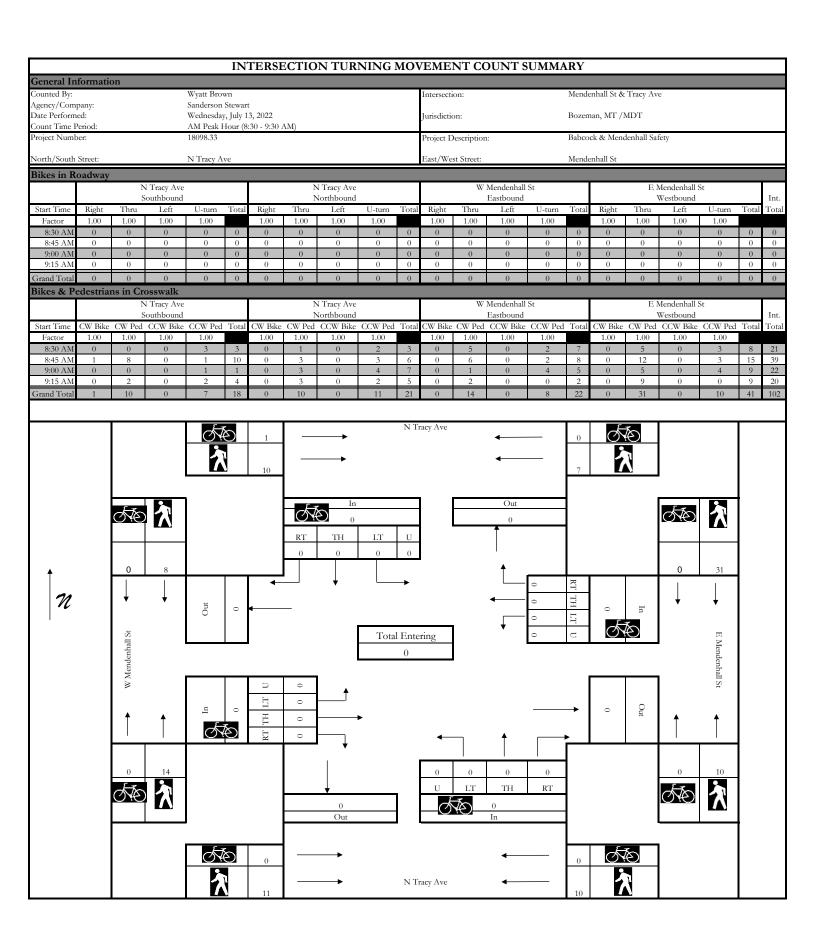




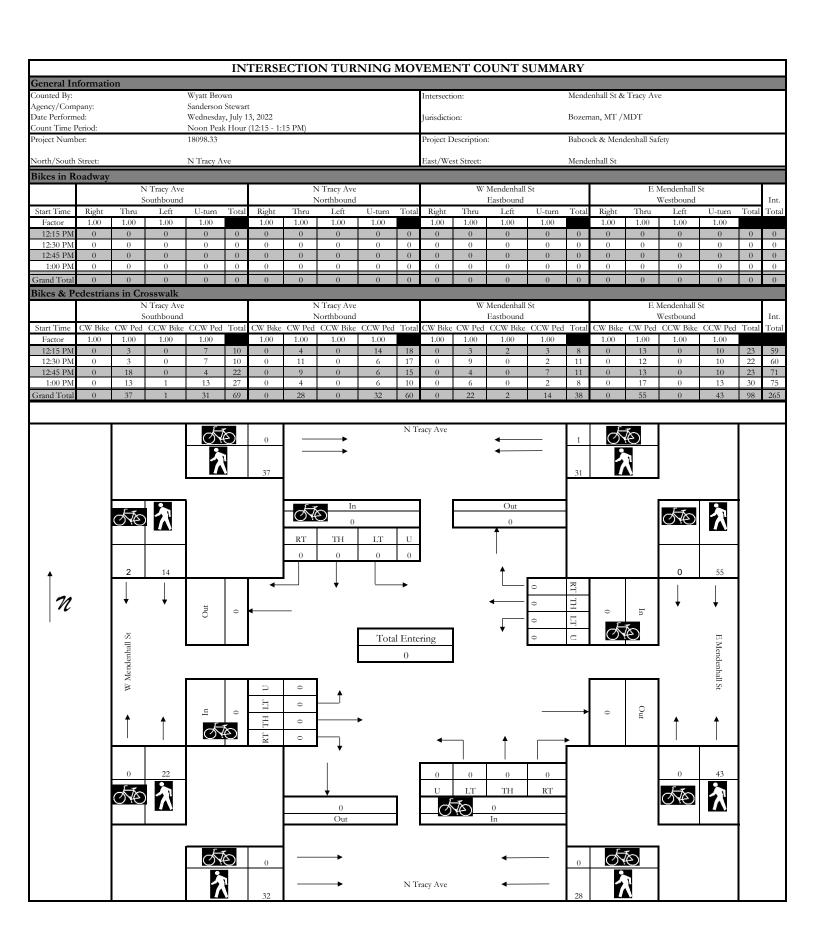




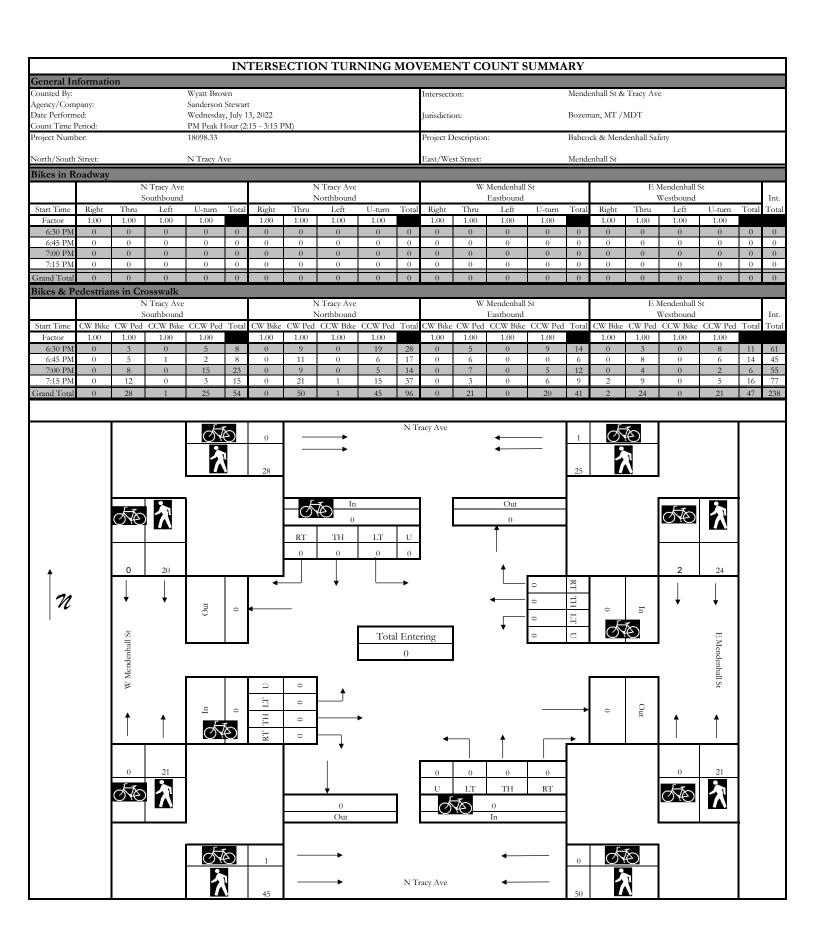




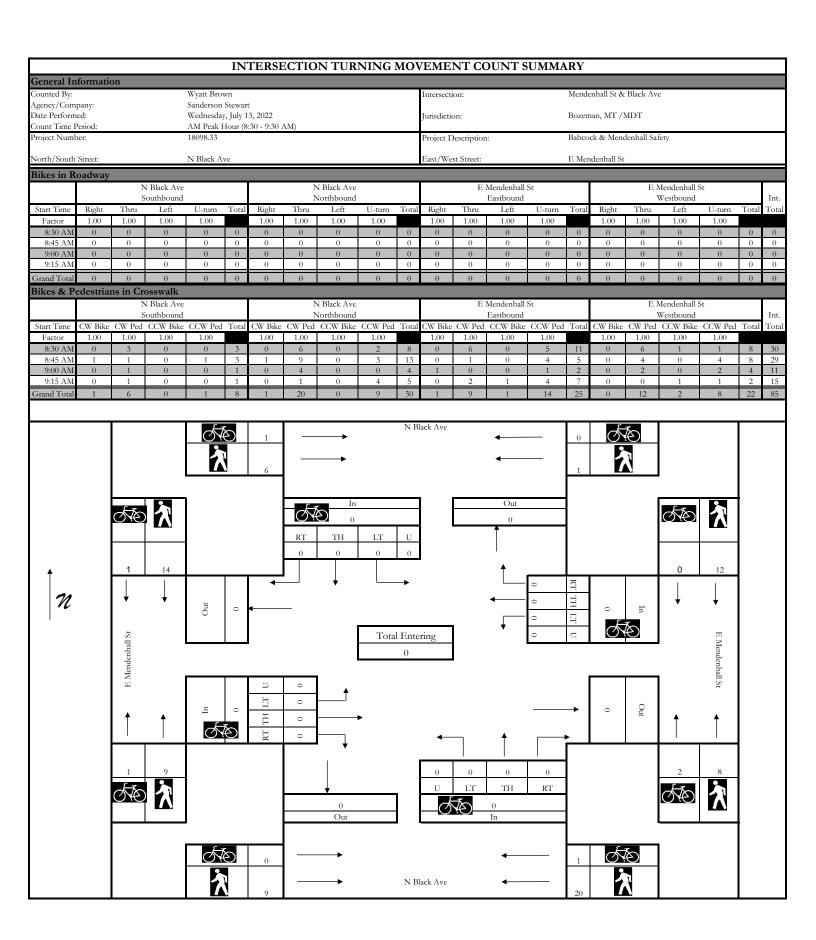




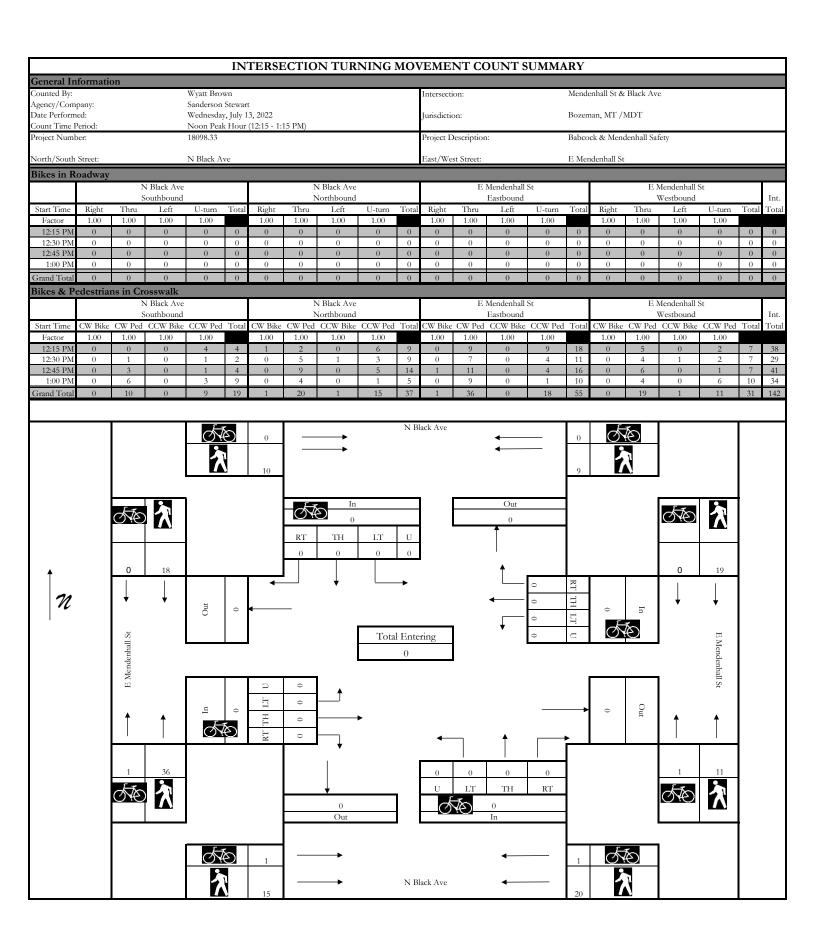




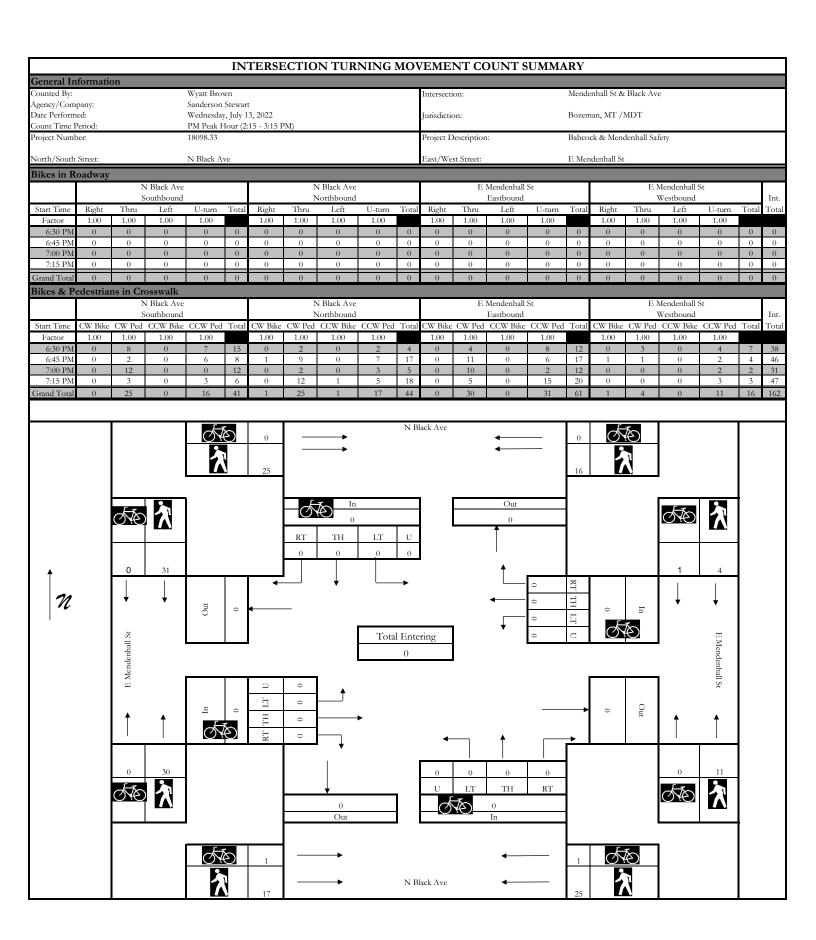




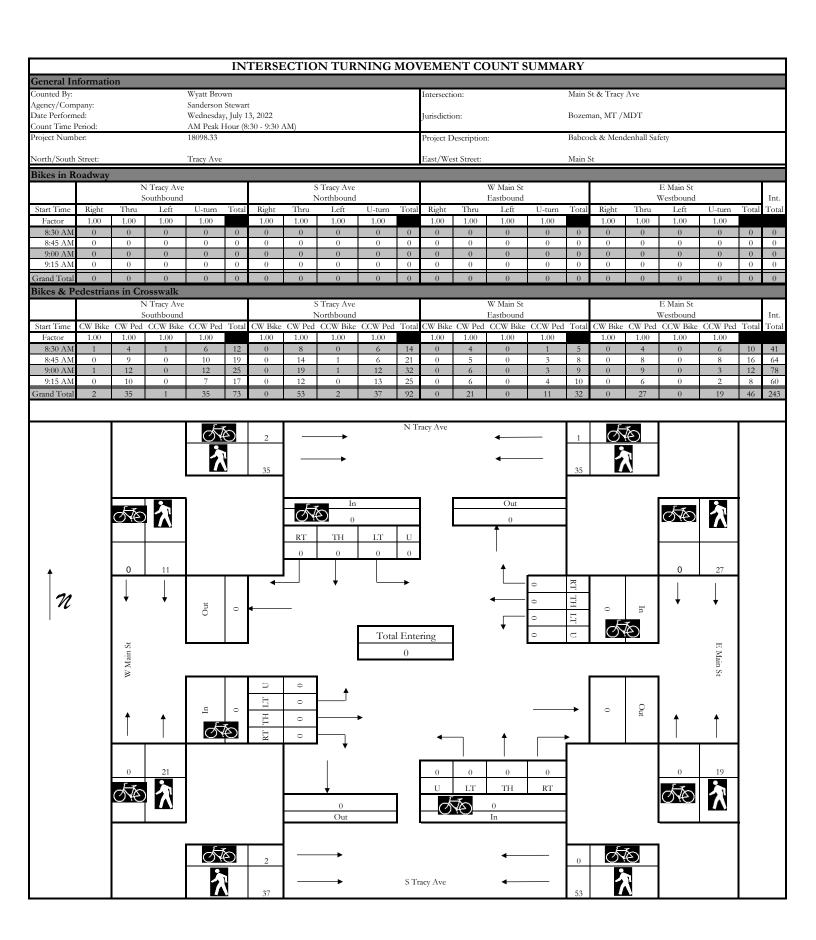




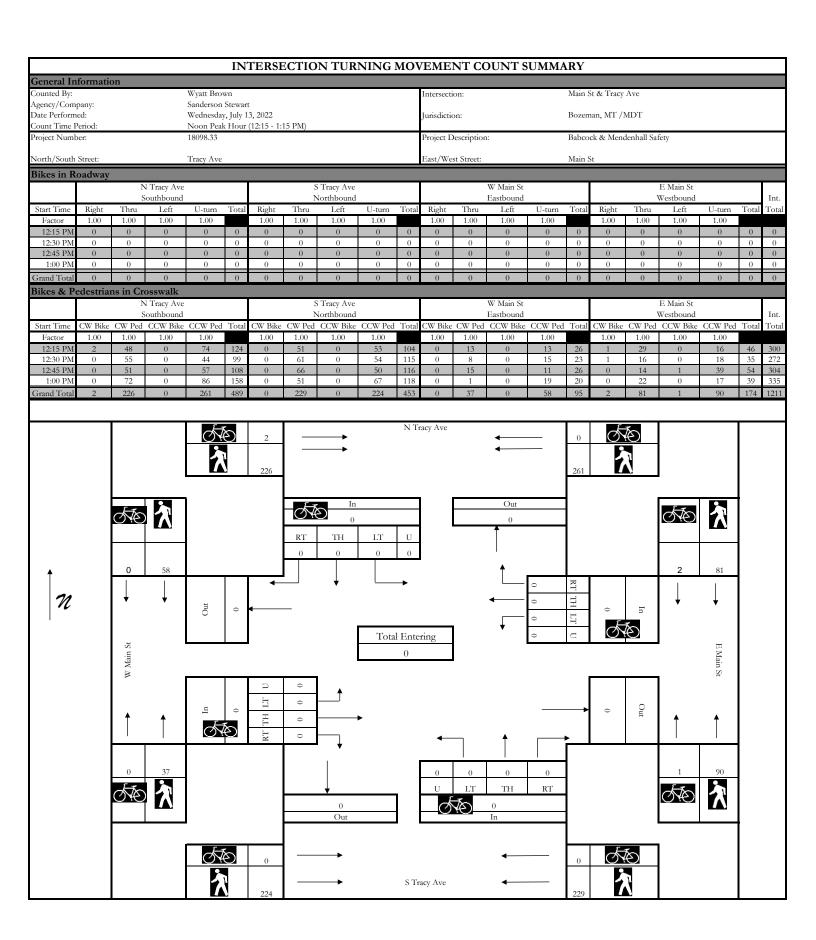




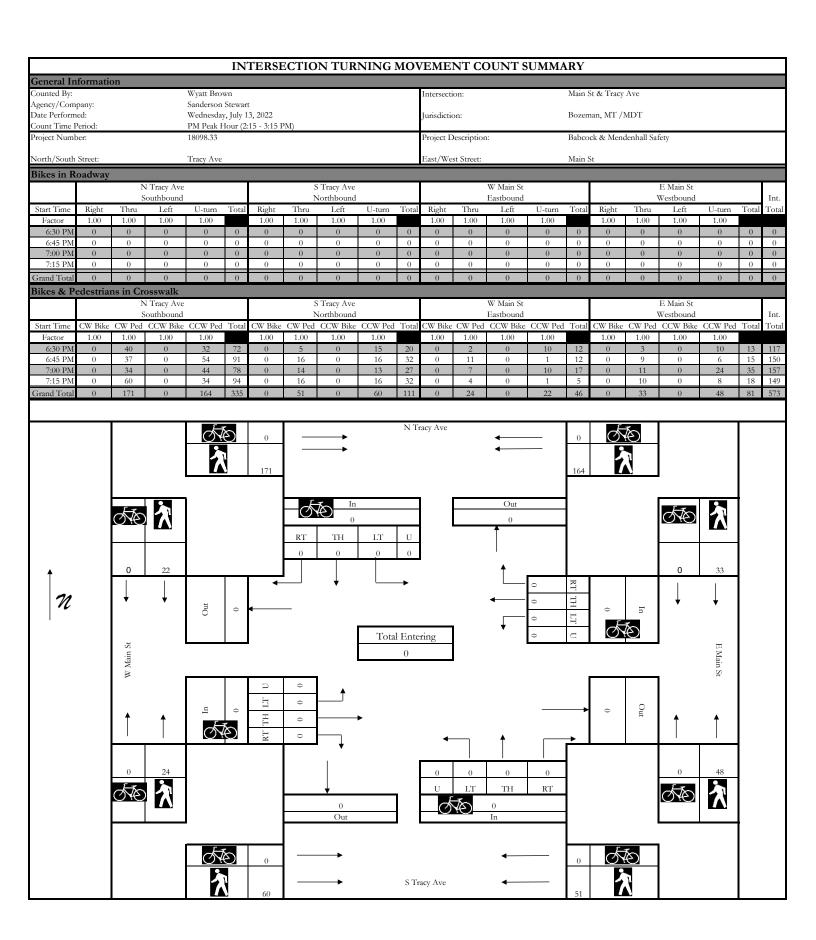




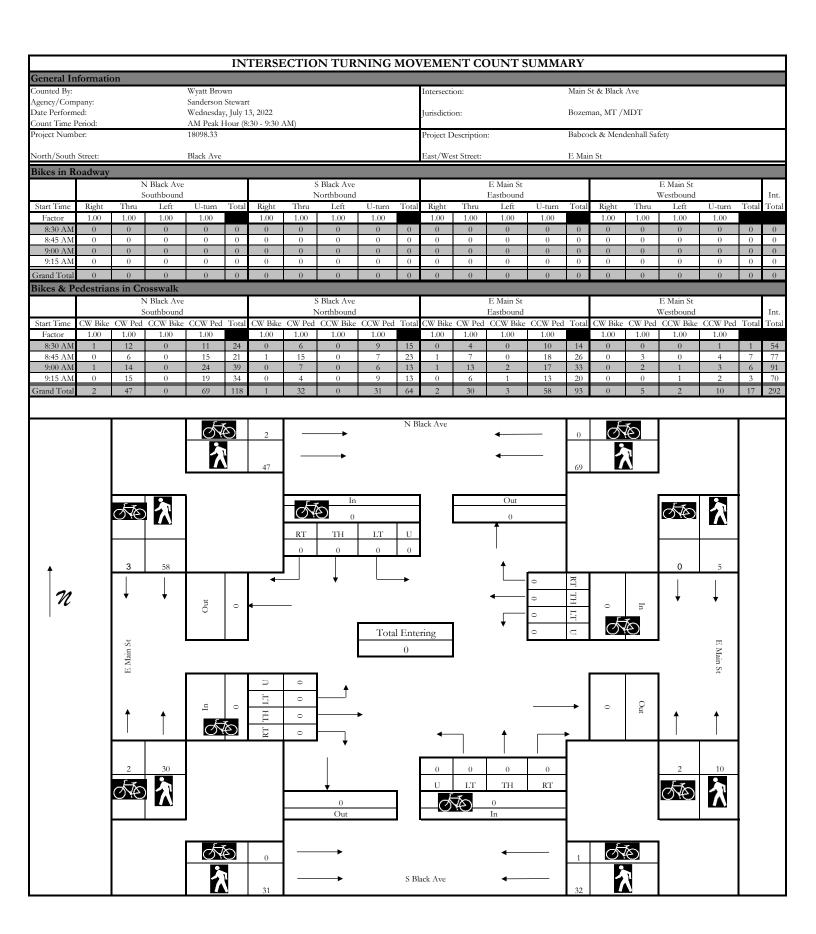




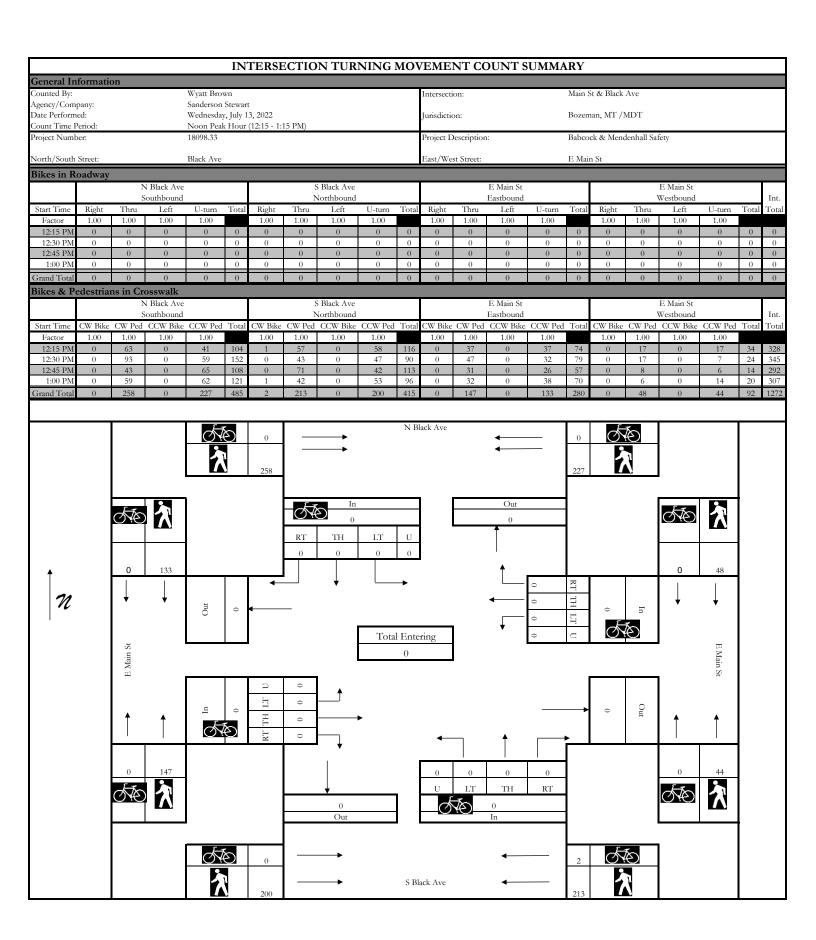




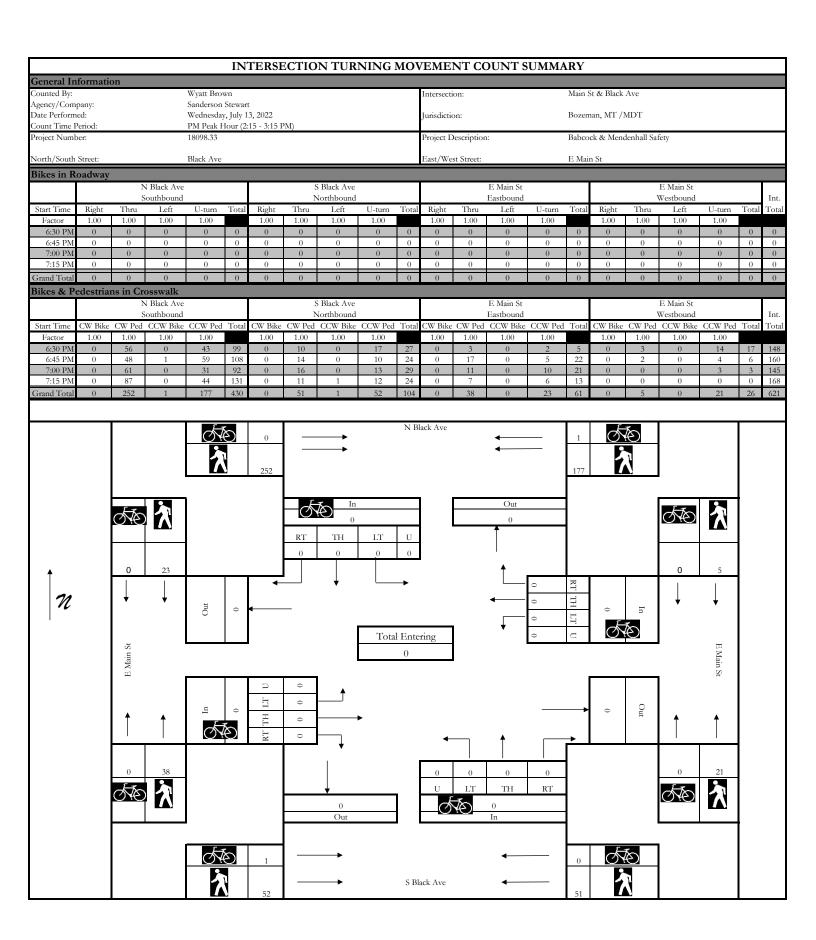




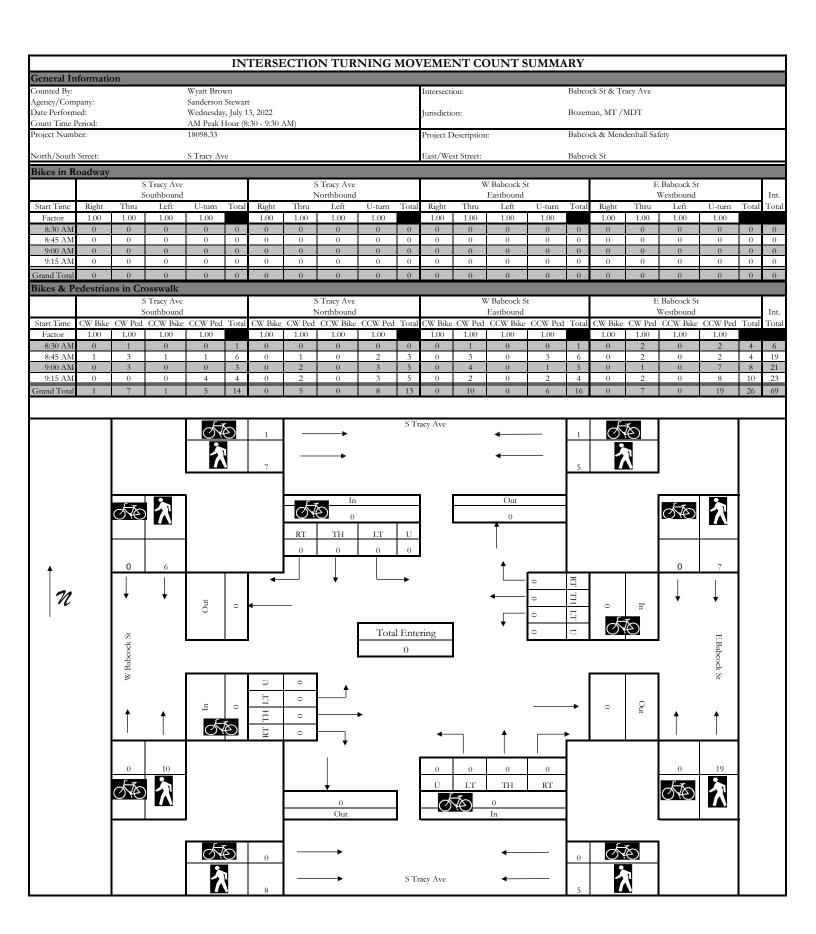




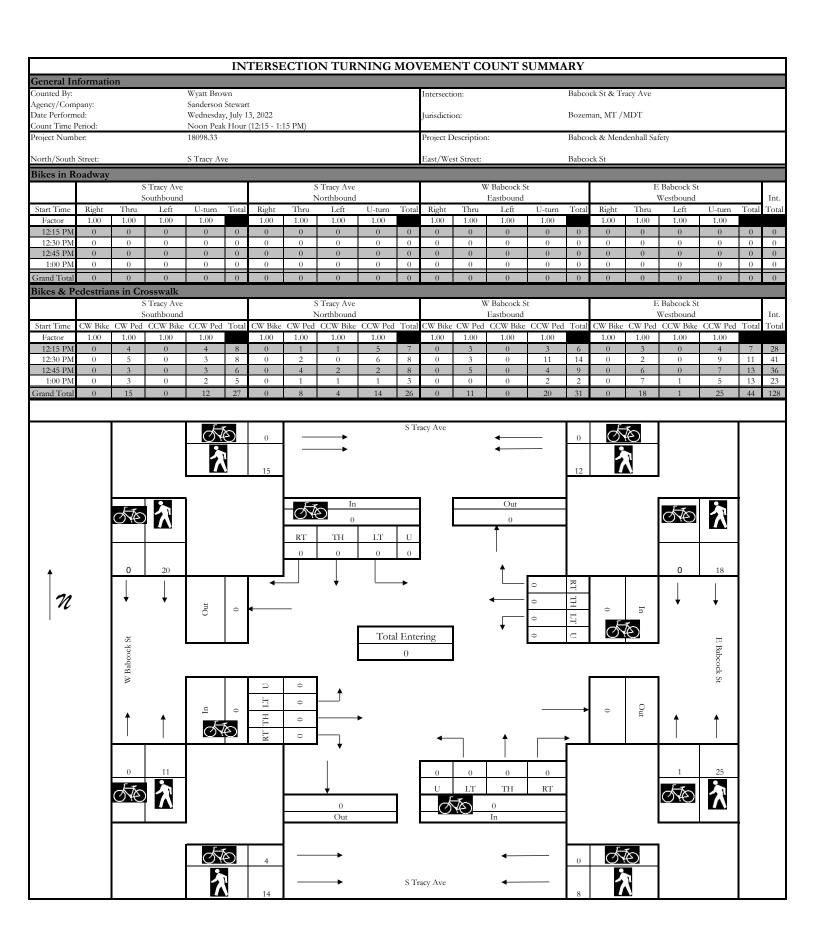




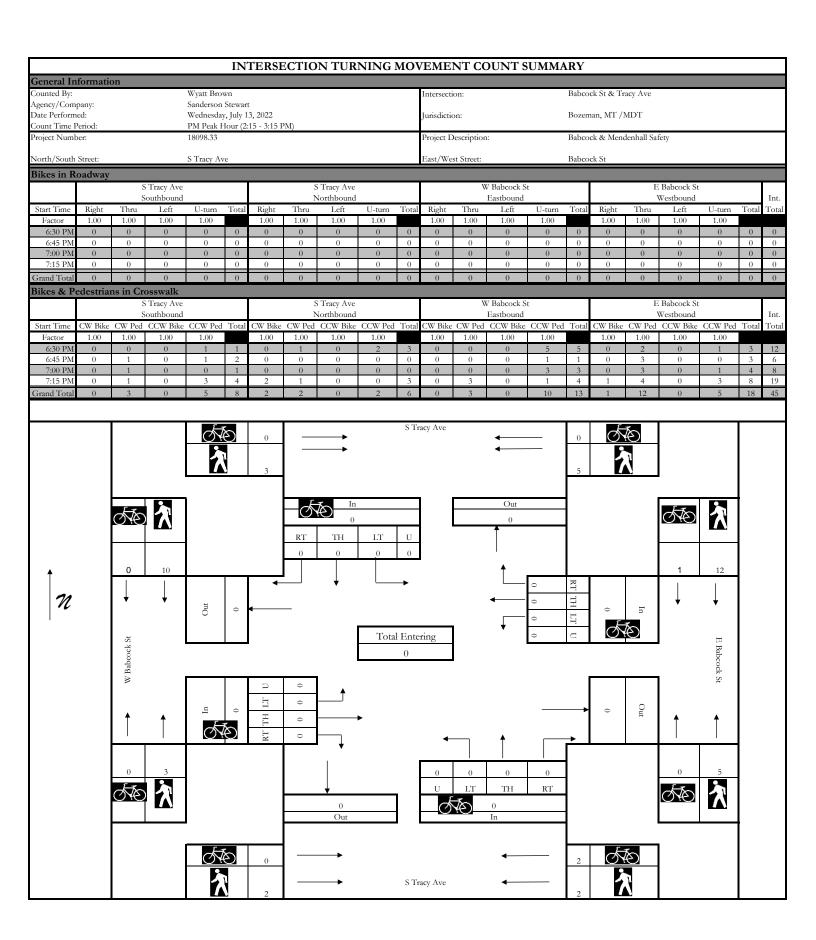




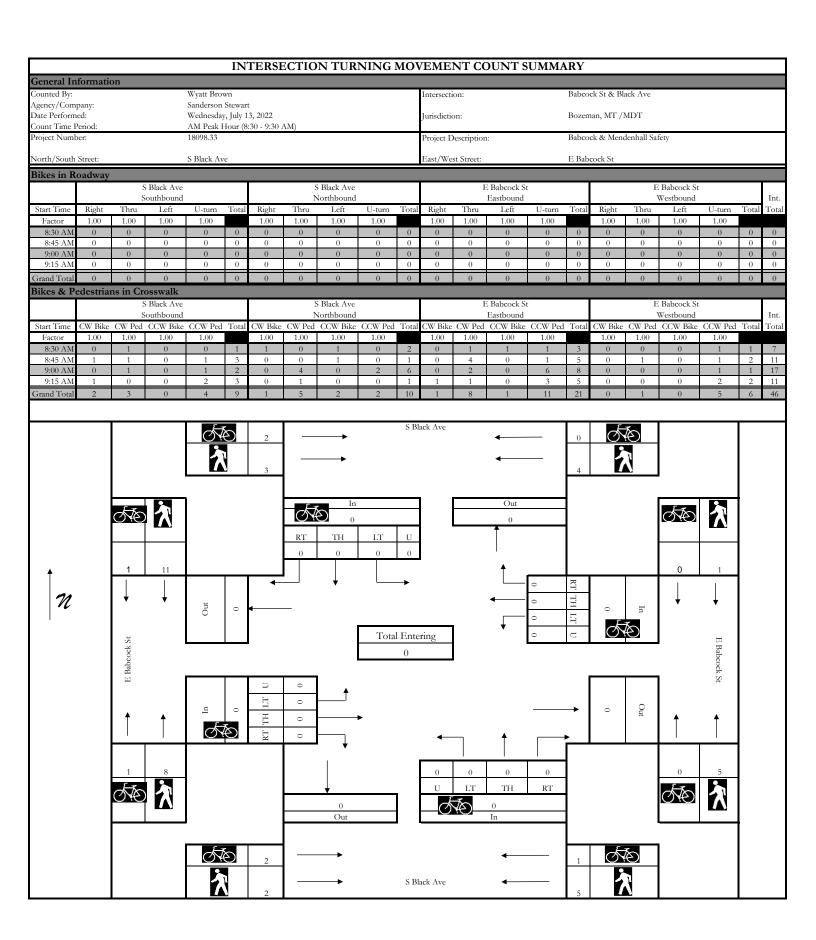




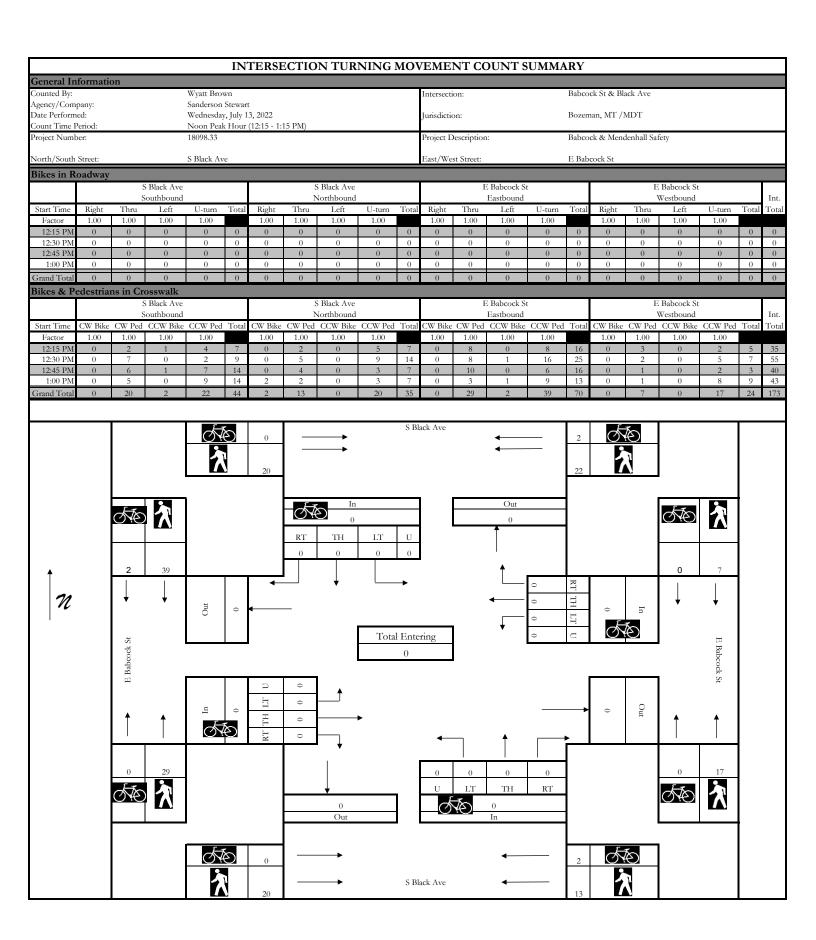




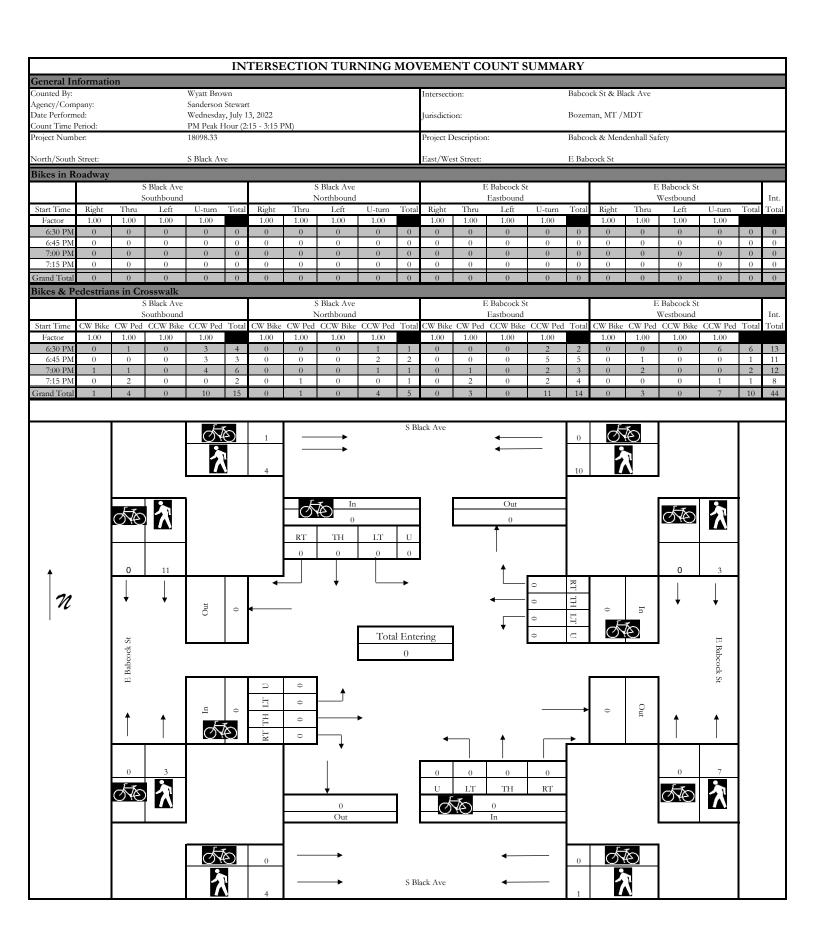
















					Exi	isting (20	)22)			
			AM Peal	X.	N	Joon Pea	ak		PM Peal	ζ
Intersection	Approach	Avg		95th %	Avg		95th %	Avg		95th %
	rr ····	Delay		Queue	Delay		Queue	Delay		Queue
		(s/veh)	LOS	(veh)	(s/veh)	LOS	(veh)	(s/veh)	LOS	(veh)
Intersection Contro	<u> </u>	(0) (01)	LOU	\ /	( '		ol (NB/S)		LOU	(**************************************
5,000,000,000	WB	0.9	Α	1	1.2	A	1	0.6	A	1
Mendenhall Street &	NB	12.5	В	1	23.4	С	2	18.2	С	1
Tracy Avenue	SB	11.8	В	1	15.2	С	1	14.9	В	1
22.02, 22.02.00	Intersection	3.7	Α		6.6	Α		4.0	А	
Intersection Contro	l			$T_{i}$	wo-Way S.	top-Contr	ol (NB/S	B)		•
	WB	0.7	Α	1	0.9	Α	1	0.7	Α	1
Mendenhall Street &	NB	13.0	В	1	17.7	С	2	17.3	С	1
Black Avenue	SB	11.1	В	1	13.2	В	1	13.8	В	1
	Intersection	4.7	Α		5.6	Α		4.4	А	
Intersection Contro	•					Signalizeo	d			
	EB	6.2	Α	2	8.4	Α	4	7.4	А	3
Main Street &	WB	0.4	Α	1	0.7	Α	2	0.5	А	2
Tracy Avenue	NB	24.3	С	2	25.2	С	3	26.3	С	3
Tracy Avenue	SB	24.7	С	3	25.9	С	4	26.3	C	3
	Intersection	6.2	А		7.9	A		7.2	А	
Intersection Contro		0.6				Signalizeo				1
	EB	0.6	A	2	0.8	A	2	0.6	A	1
Main Street &	WB	7.2	A	2	7.8	A	4	6.9	A	3
Black Avenue	NB SB	22.7	C	2 3	26.4 27.2	С	3	26.8	C	3
	-	6.3	C A		7.3	C A	4	26.9 6.8	C A	
Intersection Contro	Intersection	0.3	Λ				ol (NB/S)	0.0	Λ	
Intersection Contro	EB	0.5	A	1 0	0.6	iop-Conir A	1 1	0.5	A	1
Babcock Street &	NB	11.5	В	1	13.4	В	1	12.5	В	1
	SB	13.5	В	1	17.7	C	2	13.8	В	1
Tracy Avenue	Intersection	3.5	A		5.3	A		3.5	A	
Intersection Contro		5.5	4.1				ol (NB/S		4.1	
	EB	0.4	Α	0	0.4	A	0 (1 (1), 51	0.5	Α	1
Babcock Street &	NB	11.4	В	1	13.9	В	1	13.2	В	1
Black Avenue	SB	13.6	В	1	16.2	С	1	15.8	С	1
Diack 11 venue	Intersection	2.8	А		3.5	A		4.3	A	

# 7: Tracy Ave & Main St

	-	<b>←</b>	<b>†</b>	ļ
Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	451	389	50	73
v/c Ratio	0.20	0.17	0.29	0.41
Control Delay	3.2	2.0	26.1	30.8
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	3.2	2.0	26.1	30.8
Queue Length 50th (ft)	24	15	14	24
Queue Length 95th (ft)	48	24	43	59
Internal Link Dist (ft)	362	282	299	282
Turn Bay Length (ft)				
Base Capacity (vph)	2254	2292	435	451
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.20	0.17	0.11	0.16
Intersection Summary				

	ၨ	<b>→</b>	•	•	<b>←</b>	•	•	<b>†</b>	/	<b>&gt;</b>	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4î>			413-			4			4	
Traffic Volume (veh/h)	25	368	31	18	326	22	17	14	16	17	34	18
Future Volume (veh/h)	25	368	31	18	326	22	17	14	16	17	34	18
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	0.96		0.93	0.98		0.93	0.94		0.93	0.94		0.93
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1750	1709	1750	1750	1709	1750	1750	1654	1750	1668	1709	1668
Adj Flow Rate, veh/h	27	391	33	19	347	23	18	15	17	18	36	19
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	0	3	0	0	3	0	0	7	0	6	3	6
Cap, veh/h	134	1760	146	113	1827	119	146	113	100	110	187	84
Arrive On Green	0.64	0.64	0.64	1.00	1.00	1.00	0.21	0.21	0.21	0.21	0.21	0.21
Sat Flow, veh/h	125	2751	228	94	2855	186	383	538	474	240	889	397
Grp Volume(v), veh/h	235	0	216	203	0	186	50	0	0	73	0	0
Grp Sat Flow(s), veh/h/ln	1612	0	1493	1631	0	1505	1395	0	0	1526	0	0
Q Serve(g_s), s	0.0	0.0	4.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	4.3	0.0	4.6	0.0	0.0	0.0	2.0	0.0	0.0	2.8	0.0	0.0
Prop In Lane	0.11	0.0	0.15	0.09	0.0	0.12	0.36	0.0	0.34	0.25	0.0	0.26
Lane Grp Cap(c), veh/h	1085	0	956	1096	0	963	359	0	0.04	381	0	0.20
V/C Ratio(X)	0.22	0.00	0.23	0.19	0.00	0.19	0.14	0.00	0.00	0.19	0.00	0.00
Avail Cap(c_a), veh/h	1085	0.00	956	1096	0.00	963	492	0.00	0.00	526	0.00	0.00
HCM Platoon Ratio	1.00	1.00	1.00	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	0.99	0.00	0.99	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	5.6	0.00	5.7	0.0	0.00	0.0	24.2	0.0	0.00	24.5	0.0	0.00
Incr Delay (d2), s/veh	0.5	0.0	0.5	0.4	0.0	0.4	0.2	0.0	0.0	0.2	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.4	0.0	1.3	0.0	0.0	0.0	0.7	0.0	0.0	1.1	0.0	0.0
Unsig. Movement Delay, s/veh		0.0	1.0	0.1	0.0	0.1	0.7	0.0	0.0	1.1	0.0	0.0
LnGrp Delay(d),s/veh	6.1	0.0	6.2	0.4	0.0	0.4	24.3	0.0	0.0	24.7	0.0	0.0
LnGrp LOS	Α	Α	Α	Α	Α	Α	24.3 C	Α	Α	C C	Α	Α
								50			73	
Approach Vol, veh/h		451			389							
Approach LOS		6.2			0.4			24.3 C			24.7 C	
Approach LOS		Α			Α			C			C	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		53.3		21.7		53.3		21.7				
Change Period (Y+Rc), s		* 5.3		5.9		* 5.3		5.9				
Max Green Setting (Gmax), s		* 41		23.1		* 41		23.1				
Max Q Clear Time (g_c+I1), s		6.6		4.8		2.0		4.0				
Green Ext Time (p_c), s		3.0		0.3		2.6		0.2				
Intersection Summary												
HCM 6th Ctrl Delay			6.2									
HCM 6th LOS			Α									
Notes												

<sup>\*</sup> HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection												
Int Delay, s/veh	3.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					414			4			₽	
Traffic Vol, veh/h	0	0	0	34	257	16	25	35	0	0	23	16
Future Vol, veh/h	0	0	0	34	257	16	25	35	0	0	23	16
Conflicting Peds, #/hr	18	0	21	21	0	18	22	0	41	41	0	22
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	1	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	0	0	0	6	5	0	0	3	0	0	9	25
Mvmt Flow	0	0	0	36	271	17	26	37	0	0	24	17
Major/Minor				Maisro			ling -1			line -0		
Major/Minor				Major2			/linor1	000	<u> </u>	/linor2	001	404
Conflicting Flow All				21	0	0	263	399	-	-	391	184
Stage 1				-	-	-	21	21	-	-	370	-
Stage 2				-	-	-	242	378	-	-	21	-
Critical Hdwy				4.22	-	-	7.5	6.56	-	-	6.68	7.4
Critical Hdwy Stg 1				-	-	-	-	-	-	-	5.68	-
Critical Hdwy Stg 2				-	-	-	6.5	5.56	-	-	-	-
Follow-up Hdwy				2.26	-	-	3.5	4.03	-	-	4.09	3.55
Pot Cap-1 Maneuver				1564	-	-	674	535	0	0	528	760
Stage 1				-	-	-	-	-	0	0	601	-
Stage 2				-	-	-	746	611	0	0	-	-
Platoon blocked, %					-	-						
Mov Cap-1 Maneuver				1533	-	-	608	501	-	-	494	747
Mov Cap-2 Maneuver				-	-	-	608	501	-	-	494	-
Stage 1				-	-	-	-	-	-	-	574	-
Stage 2				-	-	-	679	584	-	-	-	-
Approach				WB			NB			SB		
HCM Control Delay, s				0.9			12.5			11.8		
HCM LOS				3.0			В			В		
Minor Lane/Major Mvmt	: 1	NBLn1	WBL	WBT	WBR	SBLn1						
Capacity (veh/h)		541	1533	_	_	574						
HCM Lane V/C Ratio		0.117		_	_	0.072						
HCM Control Delay (s)		12.5	7.4	0.1	_	11.8						
HCM Lane LOS		12.3 B	Α	A	_	В						
HCM 95th %tile Q(veh)		0.4	0.1	-		0.2						
HOW JOHN JOHN Q(VEH)		0.4	0.1	_		0.2						

Intersection												
Int Delay, s/veh	3.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		414						ĵ.			4	
Traffic Vol, veh/h	19	352	14	0	0	0	0	15	28	60	23	0
Future Vol, veh/h	19	352	14	0	0	0	0	15	28	60	23	0
Conflicting Peds, #/hr	14	0	13	13	0	14	16	0	26	26	0	16
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	83	83	83	83	83	83	83	83	83	83	83	83
Heavy Vehicles, %	0	2	0	0	0	0	0	0	0	2	0	0
Mvmt Flow	23	424	17	0	0	0	0	18	34	72	28	0
Major/Minor N	/lajor1					N	/linor1		N	/linor2		
Conflicting Flow All	14	0	0				-	506	260	307	514	-
Stage 1	-	-	-				-	492	-	14	14	-
Stage 2	-	-	-				-	14	-	293	500	-
Critical Hdwy	4.1	-	-				-	6.5	6.9	7.54	6.5	-
Critical Hdwy Stg 1	-	-	-				-	5.5	-	-	-	-
Critical Hdwy Stg 2	-	-	-				-	-	-	6.54	5.5	-
Follow-up Hdwy	2.2	-	-				-	4	3.3	3.52	4	-
Pot Cap-1 Maneuver	1617	-	-				0	472	745	622	467	0
Stage 1	-	-	-				0	551	-	-	-	0
Stage 2	-	_	-				0	-	-	691	546	0
Platoon blocked, %		-	-									
Mov Cap-1 Maneuver	1595	-	-				-	452	736	560	447	-
Mov Cap-2 Maneuver	-	-	-				-	452	-	560	447	-
Stage 1	-	-	-				-	534	-	-	-	-
Stage 2	-	-	-				-	-	-	625	529	-
Approach	EB						NB			SB		
HCM Control Delay, s	0.5						11.5			13.5		
HCM LOS							В			В		
Minor Lane/Major Mvmt	tI	NBLn1	EBL	EBT	EBR	SBLn1						
Capacity (veh/h)		604	1595		-	523						
HCM Lane V/C Ratio			0.014	-	_	0.191						
HCM Control Delay (s)		11.5	7.3	0.1	-	13.5						
HCM Lane LOS		В	A	Α	-	В						
HCM 95th %tile Q(veh)		0.3	0	-	_	0.7						
,												

# 12: Black Ave & Main St

	-	<b>←</b>	<b>†</b>	ļ
Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	470	449	60	80
v/c Ratio	0.22	0.19	0.33	0.43
Control Delay	2.4	3.3	26.5	30.9
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	2.4	3.3	26.5	30.9
Queue Length 50th (ft)	19	25	17	26
Queue Length 95th (ft)	32	47	48	62
Internal Link Dist (ft)	282	376	300	296
Turn Bay Length (ft)				
Base Capacity (vph)	2154	2312	413	424
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.22	0.19	0.15	0.19
Intersection Summary				

	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<i>&gt;</i>	<b>&gt;</b>	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्सी			€ि			4			4	
Traffic Volume (veh/h)	38	351	29	14	362	23	18	18	18	18	36	18
Future Volume (veh/h)	38	351	29	14	362	23	18	18	18	18	36	18
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	0.97		0.90	0.95		0.90	0.90		0.88	0.90		0.88
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1750	1709	1750	1750	1723	1750	1750	1750	1750	1750	1750	1750
Adj Flow Rate, veh/h	43	394	33	16	407	26	20	20	20	20	40	20
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Percent Heavy Veh, %	0	3	0	0	2	0	0	0	0	0	0	0
Cap, veh/h	185	1577	131	86	1792	112	152	142	115	121	212	91
Arrive On Green	1.00	1.00	1.00	0.61	0.61	0.61	0.24	0.24	0.24	0.24	0.24	0.24
Sat Flow, veh/h	211	2590	215	57	2941	184	365	592	478	255	884	380
Grp Volume(v), veh/h	241	0	229	236	0	213	60	0	0	80	0	0
Grp Sat Flow(s), veh/h/ln	1526	0	1489	1672	0	1511	1435	0	0	1519	0	0
Q Serve(g_s), s	0.0	0.0	0.0	0.0	0.0	4.8	0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	0.0	0.0	0.0	4.7	0.0	4.8	2.3	0.0	0.0	3.0	0.0	0.0
Prop In Lane	0.18	0.0	0.14	0.07	0.0	0.12	0.33	0.0	0.33	0.25	0.0	0.25
Lane Grp Cap(c), veh/h	986	0	907	1069	0	920	409	0	0.00	425	0	0.20
V/C Ratio(X)	0.24	0.00	0.25	0.22	0.00	0.23	0.15	0.00	0.00	0.19	0.00	0.00
Avail Cap(c_a), veh/h	986	0.00	907	1069	0.00	920	464	0.00	0.00	484	0.00	0.00
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.99	0.00	0.99	1.00	0.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	0.0	0.0	0.00	6.6	0.0	6.7	22.5	0.0	0.0	22.8	0.0	0.00
Incr Delay (d2), s/veh	0.6	0.0	0.7	0.5	0.0	0.6	0.2	0.0	0.0	0.2	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	0.0	0.0	1.6	0.0	1.5	0.8	0.0	0.0	1.1	0.0	0.0
Unsig. Movement Delay, s/veh		0.0	0.2	1.0	0.0	1.0	0.0	0.0	0.0	1.1	0.0	0.0
LnGrp Delay(d),s/veh	0.6	0.0	0.7	7.1	0.0	7.3	22.7	0.0	0.0	23.0	0.0	0.0
LnGrp LOS	Α	Α	Α	Α	Α	7.5 A	C	Α	Α	23.0 C	Α	Α
		470			449			60			80	
Approach Vol, veh/h		0.6			7.2			22.7			23.0	
Approach LOS												
Approach LOS		Α			Α			С			С	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		51.0		24.0		51.0		24.0				
Change Period (Y+Rc), s		* 5.3		6.0		* 5.3		6.0				
Max Green Setting (Gmax), s		* 43		21.0		* 43		21.0				
Max Q Clear Time (g_c+I1), s		2.0		5.0		6.8		4.3				
Green Ext Time (p_c), s		3.3		0.3		3.0		0.2				
Intersection Summary												
HCM 6th Ctrl Delay			6.3									
HCM 6th LOS			Α									
Notos												

<sup>\*</sup> HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection												
Int Delay, s/veh	4.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					414			4			<b>1</b>	
Traffic Vol, veh/h	0	0	0	23	252	13	28	42	0	0	28	62
Future Vol, veh/h	0	0	0	23	252	13	28	42	0	0	28	62
Conflicting Peds, #/hr	8	0	30	30	0	8	25	0	22	22	0	25
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	_	_	-	_	_	-	_	_	-	_	_	-
Veh in Median Storage,	# -	1	_	_	0	_	_	0	_	_	0	_
Grade, %	<i>"</i> -	0	_	_	0	_	_	0	_	_	0	_
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	0	0	0	5	8	4	2	0	0	0	3
Mvmt Flow	0	0	0	26	286	15	32	48	0	0	32	70
Major/Minor				Major2		N	/linor1		N	/linor2		
Conflicting Flow All				30	0	0	266	391		-	384	184
Stage 1				-	-	-	30	30	_	_	354	-
Stage 2				_	<u>-</u>	_	236	361	<u>-</u>	_	30	_
Critical Hdwy				4.1	_	_	7.58	6.54	_	_	6.5	6.96
Critical Hdwy Stg 1				-	_	_	- 1.00	- 0.0 1	_	_	5.5	-
Critical Hdwy Stg 2				_	_	_	6.58	5.54	-	_	-	_
Follow-up Hdwy				2.2	_	_	3.54	4.02	_	_	4	3.33
Pot Cap-1 Maneuver				1596	_	_	660	543	0	0	553	824
Stage 1				-	_	_	-	-	0	0	634	-
Stage 2				-	-	-	740	624	0	0	-	-
Platoon blocked, %					_	-						
Mov Cap-1 Maneuver				1550	_	-	550	513	-	-	522	818
Mov Cap-2 Maneuver					-	-	550	513	-	-	522	-
Stage 1				-	-	-	-	-	-	_	616	-
Stage 2				-	-	-	629	607	-	-	-	-
<b>5</b>												
Approach				WB			NB			SB		
HCM Control Delay, s				0.7			13			11.1		
HCM LOS				<b>J</b> .,			В			В		
										_		
Minor Lane/Major Mvmt	: N	NBLn1	WBL	WBT	WBR :	SBLn1						
Capacity (veh/h)			1550	-	-							
HCM Lane V/C Ratio		0.151		-		0.147						
HCM Control Delay (s)		13	7.4	0.1	_	11.1						
HCM Lane LOS		В	Α	A	_	В						
HCM 95th %tile Q(veh)		0.5	0.1	-	-	0.5						

Intersection												
Int Delay, s/veh	2.8				_							
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		414						f)			4	
Traffic Vol, veh/h	18	354	30	0	0	0	0	16	36	16	34	0
Future Vol, veh/h	18	354	30	0	0	0	0	16	36	16	34	0
Conflicting Peds, #/hr	9	0	10	10	0	9	21	0	6	6	0	21
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	83	83	83	83	83	83	83	83	83	83	83	83
Heavy Vehicles, %	0	1	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	22	427	36	0	0	0	0	19	43	19	41	0
Major/Minor N	/lajor1					N	/linor1		N	Minor2		
Conflicting Flow All	9	0	0				-	508	248	282	526	-
Stage 1	-	-	-				-	499	-	9	9	-
Stage 2	-	-	-				-	9	-	273	517	-
Critical Hdwy	4.1	-	-				-	6.5	6.9	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-				-	5.5	-	-	-	-
Critical Hdwy Stg 2	-	-	-				-	-	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-				-	4	3.3	3.5	4	-
Pot Cap-1 Maneuver	1624	-	-				0	471	758	654	460	0
Stage 1	-	-	-				0	547	-	-	-	0
Stage 2	-	-	-				0	-	-	715	537	0
Platoon blocked, %		-	-									
Mov Cap-1 Maneuver	1610	-	-				-	454	751	583	443	-
Mov Cap-2 Maneuver	-	-	-				-	454	-	583	443	-
Stage 1	-	-	-				-	532	-	-	-	-
Stage 2	-	-	-				-	-	-	638	522	-
Approach	EB						NB			SB		
HCM Control Delay, s	0.4						11.4			13.6		
HCM LOS							В			В		
Minor Lane/Major Mvmt	t N	NBLn1	EBL	EBT	EBR	SBLn1						
Capacity (veh/h)		625	1610	-	-	480						
HCM Lane V/C Ratio				-	_	0.126						
HCM Control Delay (s)		11.4	7.3	0.1	-							
HCM Lane LOS		В	A	Α	-	В						
HCM 95th %tile Q(veh)		0.3	0	-	-	0.4						

# 7: Tracy Ave & Main St

	<b>→</b>	<b>←</b>	<b>†</b>	ļ
Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	553	531	83	114
v/c Ratio	0.26	0.25	0.45	0.58
Control Delay	4.2	2.6	35.5	38.9
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	4.2	2.6	35.5	38.9
Queue Length 50th (ft)	40	26	35	48
Queue Length 95th (ft)	77	36	74	93
Internal Link Dist (ft)	362	282	299	282
Turn Bay Length (ft)				
Base Capacity (vph)	2092	2101	363	379
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.26	0.25	0.23	0.30
Intersection Summary				

	ၨ	<b>→</b>	•	•	<b>←</b>	•	•	<b>†</b>	<i>&gt;</i>	<b>&gt;</b>	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्सी∳			414			4			4	
Traffic Volume (veh/h)	24	448	53	22	441	42	29	36	13	39	42	28
Future Volume (veh/h)	24	448	53	22	441	42	29	36	13	39	42	28
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	0.91		0.82	0.96		0.82	0.86		0.83	0.85		0.83
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1695	1709	1750	1750	1695	1682	1695	1709	1750	1682	1750	1750
Adj Flow Rate, veh/h	25	472	56	23	464	44	31	38	14	41	44	29
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	4	3	0	0	4	5	4	3	0	5	0	0
Cap, veh/h	97	1624	188	94	1667	155	169	187	60	161	160	90
Arrive On Green	0.61	0.61	0.61	1.00	1.00	1.00	0.25	0.25	0.25	0.25	0.25	0.25
Sat Flow, veh/h	83	2644	307	79	2713	252	435	739	238	409	632	355
Grp Volume(v), veh/h	294	0	259	281	0	250	83	0	0	114	0	0
Grp Sat Flow(s), veh/h/ln	1615	0	1418	1613	0	1430	1412	0	0	1396	0	0
Q Serve(g_s), s	0.0	0.0	7.3	0.0	0.0	0.0	0.0	0.0	0.0	1.7	0.0	0.0
Cycle Q Clear(g_c), s	6.8	0.0	7.3	0.0	0.0	0.0	3.5	0.0	0.0	5.2	0.0	0.0
Prop In Lane	0.09	0.0	0.22	0.08	0.0	0.18	0.37	0.0	0.17	0.36	0.0	0.25
Lane Grp Cap(c), veh/h	1038	0	872	1037	0	879	417	0	0.17	412	0	0.23
V/C Ratio(X)	0.28	0.00	0.30	0.27	0.00	0.28	0.20	0.00	0.00	0.28	0.00	0.00
Avail Cap(c_a), veh/h	1038	0.00	872	1037	0.00	879	441	0.00	0.00	436	0.00	0.00
HCM Platoon Ratio	1.00	1.00	1.00	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	0.97	0.00	0.97	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	7.6	0.00	7.7	0.0	0.00	0.0	25.0	0.00	0.00	25.5	0.00	0.00
Incr Delay (d2), s/veh	0.7	0.0	0.9	0.6	0.0	0.0	0.2	0.0	0.0	0.4	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.9	0.0	0.0	0.0	0.2	0.0	0.0	0.4	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.4	0.0	2.2	0.0	0.0	0.0	1.3	0.0	0.0	1.9	0.0	0.0
	2.4	0.0	۷.۷	0.2	0.0	0.2	1.3	0.0	0.0	1.9	0.0	0.0
Unsig. Movement Delay, s/veh	0.2	0.0	0.0	0.6	0.0	0.0	05.0	0.0	0.0	25.0	0.0	0.0
LnGrp Delay(d),s/veh	8.3	0.0	8.6	0.6	0.0	0.8	25.2	0.0	0.0	25.9	0.0	0.0
LnGrp LOS	Α	A	Α	Α	A	Α	С	A	A	С	Α	A
Approach Vol, veh/h		553			531			83			114	
Approach Delay, s/veh		8.4			0.7			25.2			25.9	
Approach LOS		Α			Α			С			С	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		57.5		27.5		57.5		27.5				
Change Period (Y+Rc), s		* 5.3		5.9		* 5.3		5.9				
Max Green Setting (Gmax), s		* 51		23.1		* 51		23.1				
Max Q Clear Time (g_c+I1), s		9.3		7.2		2.0		5.5				
Green Ext Time (p_c), s		4.0		0.5		3.8		0.4				
Intersection Summary												
HCM 6th Ctrl Delay			7.9									
HCM 6th LOS			Α									
Notos												

<sup>\*</sup> HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection												
Int Delay, s/veh	6.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					414			र्स			ĵ.	
Traffic Vol, veh/h	0	0	0	65	398	24	58	63	0	0	31	41
Future Vol, veh/h	0	0	0	65	398	24	58	63	0	0	31	41
Conflicting Peds, #/hr	42	0	71	71	0	42	39	0	83	83	0	39
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	1	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	0	0	0	3	2	8	0	3	0	0	3	15
Mvmt Flow	0	0	0	70	428	26	62	68	0	0	33	44
Major/Minor			ľ	Major2		N	Minor1		N	/linor2		
Conflicting Flow All				71	0	0	481	707	-	-	694	308
Stage 1				_	-	-	71	71	-	_	623	-
Stage 2				-	-	-	410	636	-	-	71	-
Critical Hdwy				4.16	-	_	7.5	6.56	-	_	6.56	7.2
Critical Hdwy Stg 1				-	_	-	-	-	-	-	5.56	-
Critical Hdwy Stg 2				-	-	-	6.5	5.56	-	-	-	-
Follow-up Hdwy				2.23	-	_	3.5	4.03	_	_	4.03	3.45
Pot Cap-1 Maneuver				1520	-	-	473	357	0	0	363	651
Stage 1				-	-	-	-	-	0	0	474	-
Stage 2				-	-	-	595	468	0	0	-	-
Platoon blocked, %					-	-						
Mov Cap-1 Maneuver				1417	-	-	357	298	-	-	303	625
Mov Cap-2 Maneuver				-	-	-	357	298	-	-	303	-
Stage 1				-	-	-	-	-	-	-	425	-
Stage 2				-	-	-	476	420	-	-	-	-
Ü												
Approach				WB			NB			SB		
HCM Control Delay, s				1.2			23.4			15.2		
HCM LOS							С			С		
Minor Lane/Major Mvmt	t N	NBLn1	WBL	WBT	WBR	SBLn1						
Capacity (veh/h)			1417	-	-	429						
HCM Lane V/C Ratio		0.402		-	-	0.18						
HCM Control Delay (s)		23.4	7.7	0.2	_	15.2						
HCM Lane LOS		С	Α	A	-	С						
HCM 95th %tile Q(veh)		1.9	0.2	-	-	0.7						

Intersection												
Int Delay, s/veh	5.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		414						ĵ.			र्स	
Traffic Vol, veh/h	31	451	10	0	0	0	0	34	49	80	50	0
Future Vol, veh/h	31	451	10	0	0	0	0	34	49	80	50	0
Conflicting Peds, #/hr	33	0	22	22	0	33	38	0	35	35	0	38
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	3	1	0	0	0	0	0	3	0	0	0	0
Mvmt Flow	33	475	11	0	0	0	0	36	52	84	53	0
Major/Minor N	/lajor1					N	Minor1		N	/linor2		
Conflicting Flow All	33	0	0				-	602	300	390	607	-
Stage 1	-	-	-				-	569	-	33	33	-
Stage 2	-	-	-				-	33	-	357	574	-
Critical Hdwy	4.16	-	-				-	6.56	6.9	7.5	6.5	-
Critical Hdwy Stg 1	-	-	-				-	5.56	-	-	-	-
Critical Hdwy Stg 2	-	-	-				-	-	-	6.5	5.5	-
Follow-up Hdwy	2.23	-	-				-	4.03	3.3	3.5	4	-
Pot Cap-1 Maneuver	1570	-	-				0	410	702	548	414	0
Stage 1	-	-	-				0	502	-	-	-	0
Stage 2	-	-	-				0	-	-	639	506	0
Platoon blocked, %		-	-									
Mov Cap-1 Maneuver	1521	-	-				-	377	687	445	381	-
Mov Cap-2 Maneuver	-	-	-				-	377	-	445	381	-
Stage 1	-	-	-				-	477	-	-	-	-
Stage 2	-	-	-				-	-	-	530	481	-
Ĭ												
Approach	EB						NB			SB		
HCM Control Delay, s	0.6						13.4			17.7		
HCM LOS	3.0						В			C		
Minor Lane/Major Mvmt	t N	NBLn1	EBL	EBT	FBR	SBLn1						
Capacity (veh/h)		514			-	418						
HCM Lane V/C Ratio			0.021	_		0.327						
HCM Control Delay (s)		13.4	7.4	0.1	<u>-</u>	17.7						
HCM Lane LOS		13.4 B	7.4 A	Α	<u> </u>	C						
HCM 95th %tile Q(veh)		0.6	0.1	-	_	1.4						
HOW JOHN JOHN Q(VEII)		0.0	J. I			1.7						

## 12: Black Ave & Main St

	-	<b>←</b>	<b>†</b>	ļ
Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	570	594	76	101
v/c Ratio	0.28	0.27	0.42	0.55
Control Delay	2.7	4.1	35.3	38.4
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	2.7	4.1	35.3	38.4
Queue Length 50th (ft)	30	42	32	41
Queue Length 95th (ft)	42	80	68	85
Internal Link Dist (ft)	282	376	300	296
Turn Bay Length (ft)				
Base Capacity (vph)	2012	2228	340	340
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.28	0.27	0.22	0.30
Intersection Summary				

	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	~	<b>&gt;</b>	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्सीके			413-			4			4	
Traffic Volume (veh/h)	50	445	24	16	494	30	23	35	12	35	35	23
Future Volume (veh/h)	50	445	24	16	494	30	23	35	12	35	35	23
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	0.97		0.82	0.91		0.82	0.76		0.70	0.74		0.70
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1750	1709	1750	1750	1709	1750	1750	1750	1750	1750	1750	1750
Adj Flow Rate, veh/h	55	489	26	18	543	33	25	38	13	38	38	25
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	0	3	0	0	3	0	0	0	0	0	0	0
Cap, veh/h	195	1631	86	75	1841	110	137	184	55	148	133	73
Arrive On Green	1.00	1.00	1.00	0.63	0.63	0.63	0.23	0.23	0.23	0.23	0.23	0.23
Sat Flow, veh/h	229	2580	136	47	2912	174	345	785	233	381	566	312
Grp Volume(v), veh/h	285	0	285	314	0	280	76	0	0	101	0	0
Grp Sat Flow(s),veh/h/ln	1451	0	1495	1655	0	1478	1363	0	0	1259	0	0
Q Serve(g_s), s	0.0	0.0	0.0	0.0	0.0	7.3	0.0	0.0	0.0	1.7	0.0	0.0
Cycle Q Clear(g_c), s	0.0	0.0	0.0	7.0	0.0	7.3	3.3	0.0	0.0	5.1	0.0	0.0
Prop In Lane	0.19	0.0	0.09	0.06	0.0	0.12	0.33	0.0	0.17	0.38	0.0	0.25
Lane Grp Cap(c), veh/h	967	0	945	1091	0	934	377	0	0	354	0	0.20
V/C Ratio(X)	0.29	0.00	0.30	0.29	0.00	0.30	0.20	0.00	0.00	0.29	0.00	0.00
Avail Cap(c_a), veh/h	967	0	945	1091	0	934	392	0	0	369	0	0
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.97	0.00	0.97	1.00	0.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	0.0	0.0	0.0	7.0	0.0	7.1	26.1	0.0	0.0	26.7	0.0	0.0
Incr Delay (d2), s/veh	0.8	0.0	0.8	0.7	0.0	0.8	0.3	0.0	0.0	0.4	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	0.0	0.2	2.4	0.0	2.2	1.2	0.0	0.0	1.7	0.0	0.0
Unsig. Movement Delay, s/veh		0.0	0.2		0.0			0.0	0.0		0.0	0.0
LnGrp Delay(d),s/veh	0.8	0.0	0.8	7.7	0.0	7.9	26.4	0.0	0.0	27.2	0.0	0.0
LnGrp LOS	A	A	A	A	A	Α	C	A	A	C	A	A
Approach Vol, veh/h	, <u>, , , , , , , , , , , , , , , , , , </u>	570	, <u>, , , , , , , , , , , , , , , , , , </u>	, <u>, , , , , , , , , , , , , , , , , , </u>	594	, <u>, , , , , , , , , , , , , , , , , , </u>		76	,,		101	
Approach Delay, s/veh		0.8			7.8			26.4			27.2	
Approach LOS		Α			Α.			20.4 C			C C	
					А						U	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		59.0		26.0		59.0		26.0				
Change Period (Y+Rc), s		* 5.3		6.0		* 5.3		6.0				
Max Green Setting (Gmax), s		* 53		21.0		* 53		21.0				
Max Q Clear Time (g_c+l1), s		2.0		7.1		9.3		5.3				
Green Ext Time (p_c), s		4.3		0.4		4.3		0.3				
Intersection Summary												
HCM 6th Ctrl Delay			7.3									
HCM 6th LOS			Α									
Notes												

<sup>\*</sup> HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection												
Int Delay, s/veh	5.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					414			र्स			<del>(</del> Î	
Traffic Vol, veh/h	0	0	0	51	380	20	58	58	0	0	39	54
Future Vol, veh/h	0	0	0	51	380	20	58	58	0	0	39	54
Conflicting Peds, #/hr	18	0	30	30	0	18	58	0	29	29	0	58
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	1	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	96	96	96	96	96	96	96	96	96	96	96	96
Heavy Vehicles, %	0	0	0	0	3	10	2	2	0	0	0	2
Mvmt Flow	0	0	0	53	396	21	60	60	0	0	41	56
Major/Minor			ľ	Major2		N	Minor1		N	/linor2		
Conflicting Flow All				30	0	0	413	571	-	-	561	285
Stage 1				-	-	-	30	30	-	_	531	-
Stage 2				-	-	-	383	541	-	-	30	-
Critical Hdwy				4.1	-	-	7.54	6.54	-	_	6.5	6.94
Critical Hdwy Stg 1				-	-	-	-	-	-	-	5.5	-
Critical Hdwy Stg 2				-	-	-	6.54	5.54	-	-	-	-
Follow-up Hdwy				2.2	-	-	3.52	4.02	-	-	4	3.32
Pot Cap-1 Maneuver				1596	-	-	523	429	0	0	439	712
Stage 1				-	-	-	-	-	0	0	529	-
Stage 2				-	-	-	611	519	0	0	-	-
Platoon blocked, %					-	-						
Mov Cap-1 Maneuver				1550	-	-	416	391	-	-	400	700
Mov Cap-2 Maneuver				-	-	-	416	391	-	-	400	-
Stage 1				-	-	-	-	-	-	-	497	-
Stage 2				-	-	-	493	487	-	-	-	-
Approach				WB			NB			SB		
HCM Control Delay, s				0.9			17.7			13.2		
HCM LOS							С			В		
Minor Lane/Major Mvmt	t <u></u> N	NBLn1	WBL	WBT	WBR	SBLn1						
Capacity (veh/h)		403	1550	-	-	533						
HCM Lane V/C Ratio		0.3	0.034	-	-	0.182						
HCM Control Delay (s)		17.7	7.4	0.1	-							
HCM Lane LOS		С	Α	Α	-	В						
HCM 95th %tile Q(veh)		1.2	0.1	-	-	0.7						
•												

Int Delay, s/veh 3.5			
M I FDI FDI MDI MDI MDI NDI NDI NDI			
Movement EBL EBT EBR WBL WBT WBR NBL NBT N	NBR SBL	SBT	SBR
Lane Configurations 13		ર્ન	
Traffic Vol, veh/h 24 459 57 0 0 0 36	37 26	45	0
Future Vol, veh/h 24 459 57 0 0 0 36	37 26	45	0
Conflicting Peds, #/hr 33 0 34 34 0 33 91 0	18 18	0	91
	Stop Stop	Stop	Stop
	Vone -	-	None
Storage Length		-	-
Veh in Median Storage, # - 0 0 0		0	-
Grade, % - 0 0		0	-
Peak Hour Factor 97 97 97 97 97 97 97	97 97	97	97
Heavy Vehicles, % 0 1 0 0 0 0 0 0	0 0	0	0
Mvmt Flow 25 473 59 0 0 0 37	38 27	46	0
Major/Minor Major1 Minor1	Minor2		
	318 356	649	_
Stage 1 587	- 33	33	_
Stage 2 33	- 323	616	_
<u> </u>	6.9 7.5	6.5	-
Critical Hdwy Stg 1 5.5		-	_
Critical Hdwy Stg 2	- 6.5	5.5	-
	3.3 3.5	4	_
	684 580	391	0
Stage 1 0 500		-	0
Stage 2 0 -	- 669	485	0
Platoon blocked, %			
- <u></u>	662 481	358	-
Mov Cap-2 Maneuver 373	- 481	358	-
Stage 1 473		-	-
Stage 2	- 568	459	-
Approach EB NB	SB		
	16.2		
•	16.2 C		
HCM LOS B	U		
M' I M' M ( NDI 4 ED) EDT EDD OD 4			
Minor Lane/Major Mvmt NBLn1 EBL EBT EBR SBLn1			
Capacity (veh/h) 479 1542 395			
HCM Lane V/C Ratio 0.157 0.016 0.185			
HCM Control Delay (s) 13.9 7.4 0.1 - 16.2			
HCM Lane LOS B A A - C			
HCM 95th %tile Q(veh) 0.6 0 0.7			

# 7: Tracy Ave & Main St

	<b>→</b>	<b>←</b>	<b>†</b>	ļ
Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	524	445	73	75
v/c Ratio	0.23	0.19	0.44	0.43
Control Delay	3.3	2.4	37.3	32.8
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	3.3	2.4	37.3	32.8
Queue Length 50th (ft)	32	16	30	26
Queue Length 95th (ft)	60	35	68	64
Internal Link Dist (ft)	362	282	299	282
Turn Bay Length (ft)				
Base Capacity (vph)	2286	2404	389	405
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.23	0.19	0.19	0.19
Intersection Summary				

	•	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	~	<b>&gt;</b>	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्सी			414			4			44	
Traffic Volume (veh/h)	24	461	29	11	411	15	31	26	14	23	28	23
Future Volume (veh/h)	24	461	29	11	411	15	31	26	14	23	28	23
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	0.92		0.85	0.96		0.85	0.91		0.89	0.90		0.89
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1750	1723	1750	1750	1736	1750	1750	1750	1559	1695	1750	1750
Adj Flow Rate, veh/h	24	470	30	11	419	15	32	27	14	23	29	23
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	0	2	0	0	1	0	0	0	14	4	0	0
Cap, veh/h	103	1813	114	67	1959	69	188	148	65	136	159	106
Arrive On Green	0.63	0.63	0.63	1.00	1.00	1.00	0.23	0.23	0.23	0.23	0.23	0.23
Sat Flow, veh/h	90	2862	180	35	3094	109	543	628	278	343	675	450
Grp Volume(v), veh/h	274	0	250	234	0	211	73	0	0	75	0	0
Grp Sat Flow(s), veh/h/ln	1636	0	1496	1701	0	1537	1449	0	0	1468	0	0
Q Serve(g_s), s	0.0	0.0	6.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	5.9	0.0	6.2	0.0	0.0	0.0	3.0	0.0	0.0	3.2	0.0	0.0
Prop In Lane	0.09	0.0	0.12	0.05	0.0	0.07	0.44	0.0	0.19	0.31	0.0	0.31
Lane Grp Cap(c), veh/h	1082	0	948	1122	0	973	401	0	0.13	400	0	0.51
V/C Ratio(X)	0.25	0.00	0.26	0.21	0.00	0.22	0.18	0.00	0.00	0.19	0.00	0.00
Avail Cap(c_a), veh/h	1082	0.00	948	1122	0.00	973	453	0.00	0.00	453	0.00	0.00
HCM Platoon Ratio	1.00	1.00	1.00	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	0.99	0.00	0.99	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	6.8	0.00	6.9	0.0	0.00	0.0	26.0	0.00	0.00	26.1	0.00	0.00
Incr Delay (d2), s/veh	0.6	0.0	0.3	0.4	0.0	0.5	0.2	0.0	0.0	0.2	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.4	0.0	0.0	0.2	0.0	0.0	0.2	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.1	0.0	1.9	0.0	0.0	0.0	1.2	0.0	0.0	1.2	0.0	0.0
		0.0	1.9	0.1	0.0	0.1	1.2	0.0	0.0	1.2	0.0	0.0
Unsig. Movement Delay, s/veh		0.0	7.5	0.4	0.0	0.5	26.3	0.0	0.0	26.3	0.0	0.0
LnGrp Delay(d),s/veh	7.4						20.3 C					
LnGrp LOS	A	A	A	A	A	A	<u> </u>	A	A	С	A	A
Approach Vol, veh/h		524			445			73			75	
Approach Delay, s/veh		7.4			0.5			26.3			26.3	
Approach LOS		Α			Α			С			С	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		59.1		25.9		59.1		25.9				
Change Period (Y+Rc), s		* 5.3		5.9		* 5.3		5.9				
Max Green Setting (Gmax), s		* 51		23.1		* 51		23.1				
Max Q Clear Time (g_c+l1), s		8.2		5.2		2.0		5.0				
Green Ext Time (p_c), s		3.7		0.3		3.0		0.3				
Intersection Summary												
HCM 6th Ctrl Delay			7.2									
HCM 6th LOS			Α									
Notes												

<sup>\*</sup> HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection												
Int Delay, s/veh	4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4î.			र्स			f)	
Traffic Vol, veh/h	0	0	0	35	454	21	38	39	0	0	30	29
Future Vol, veh/h	0	0	0	35	454	21	38	39	0	0	30	29
Conflicting Peds, #/hr	29	0	53	53	0	29	31	0	52	52	0	31
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	1	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	0	0	3	2	5	0	0	0	0	0	10
Mvmt Flow	0	0	0	40	516	24	43	44	0	0	34	33
Major/Minor			ľ	Major2		1	Minor1		N	/linor2		
Conflicting Flow All				53	0	0	439	702	-	-	690	330
Stage 1				-	-	_	53	53	_	-	637	-
Stage 2				_	-	_	386	649	_	-	53	-
Critical Hdwy				4.16	-	-	7.5	6.5	-	-	6.5	7.1
Critical Hdwy Stg 1				-	-	-	-	-	-	-	5.5	-
Critical Hdwy Stg 2				-	-	-	6.5	5.5	-	-	-	-
Follow-up Hdwy				2.23	-	-	3.5	4	-	-	4	3.4
Pot Cap-1 Maneuver				1544	-	-	506	365	0	0	371	643
Stage 1				-	-	-	-	-	0	0	475	-
Stage 2				-	-	-	614	469	0	0	-	-
Platoon blocked, %					-	-						
Mov Cap-1 Maneuver				1466	-	-	407	324	-	-	329	625
Mov Cap-2 Maneuver				-	-	-	407	324	-	-	329	-
Stage 1				-	-	-	-	-	-	-	444	-
Stage 2				-	-	-	516	438	-	-	-	-
Approach				WB			NB			SB		
HCM Control Delay, s				0.6			18.2			14.9		
HCM LOS							С			В		
							-					
Minor Lane/Major Mvmt	t N	NBLn1	WBL	WBT	WBR	SBLn1						
Capacity (veh/h)			1466	-	-	429						
HCM Lane V/C Ratio		0.243		_		0.156						
HCM Control Delay (s)		18.2	7.5	0.1	-							
HCM Lane LOS		С	A	A	-	В						
HCM 95th %tile Q(veh)		0.9	0.1	-	-	0.5						

Intersection												
Int Delay, s/veh	3.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		414						f)			4	
Traffic Vol, veh/h	25	365	14	0	0	0	0	31	22	37	35	0
Future Vol, veh/h	25	365	14	0	0	0	0	31	22	37	35	0
Conflicting Peds, #/hr	9	0	10	10	0	9	25	0	21	21	0	25
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	4	1	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	29	424	16	0	0	0	0	36	26	43	41	0
Major/Minor N	Major1					N	/linor1		N	Minor2		
Conflicting Flow All	9	0	0				-	509	251	318	517	
Stage 1	-	-	-				_	500	201	9	9	_
Stage 2	_	_	_				_	9	<u>-</u>	309	508	
Critical Hdwy	4.18	_	-				-	6.5	6.9	7.5	6.5	_
Critical Hdwy Stg 1	7.10	_	_				_	5.5	0.9	7.5	0.5	_
Critical Hdwy Stg 2	_	_	-				_	5.5	<u>-</u>	6.5	5.5	_
Follow-up Hdwy	2.24	-	-				-	4	3.3	3.5	3.5	_
Pot Cap-1 Maneuver	1595	_					0	470	755	616	465	0
Stage 1	1090	_	_				0	546	755	-	405	0
Stage 2		-	-				0	540	-	682	542	0
Platoon blocked, %	_	-	-				U	-	-	002	J4Z	U
Mov Cap-1 Maneuver	1581	_	-				_	450	748	544	445	_
Mov Cap-1 Maneuver	1301	_	_				_	450	740	544	445	_
Stage 1		-	-				-	527	-	544	445	_
Stage 1 Stage 2	-	-	_				-	JZI	-	599	524	-
Slaye Z	_	-	_				-	_	_	JJJ	J24	-
Approach	EB						NB			SB		
HCM Control Delay, s	0.5						12.5			13.8		
HCM LOS							В			В		
Minor Lane/Major Mvm	t 1	NBLn1	EBL	EBT	EBR :	SBLn1						
Capacity (veh/h)		539	1581	-	-	491						
HCM Lane V/C Ratio		0.114		-	-	0.171						
HCM Control Delay (s)		12.5	7.3	0.1	_	13.8						
HCM Lane LOS		В	A	A	_	В						
HCM 95th %tile Q(veh)		0.4	0.1	-	-	0.6						
			•••									

# 12: Black Ave & Main St

	-	<b>←</b>	<b>†</b>	ļ
Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	492	464	73	76
v/c Ratio	0.24	0.19	0.45	0.44
Control Delay	1.8	3.3	38.7	33.5
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	1.8	3.3	38.7	33.5
Queue Length 50th (ft)	16	28	31	27
Queue Length 95th (ft)	24	54	69	65
Internal Link Dist (ft)	282	376	300	296
Turn Bay Length (ft)				
Base Capacity (vph)	2091	2418	339	346
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.24	0.19	0.22	0.22
Intersection Summary				

	۶	<b>→</b>	•	•	<b>←</b>	4	4	<b>†</b>	<b>/</b>	<b>/</b>	Ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>€</b> ₽			414			4			4	
Traffic Volume (veh/h)	46	403	33	11	430	14	36	24	12	28	24	23
Future Volume (veh/h)	46	403	33	11	430	14	36	24	12	28	24	23
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	0.95		0.85	0.92		0.85	0.85		0.83	0.85		0.83
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1750	1709	1750	1750	1750	1750	1750	1750	1750	1654	1750	1750
Adj Flow Rate, veh/h	47	411	34	11	439	14	37	24	12	29	24	23
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	0	3	0	0	0	0	0	0	0	7	0	0
Cap, veh/h	197	1618	133	66	2005	63	204	122	52	155	121	95
Arrive On Green	1.00	1.00	1.00	0.64	0.64	0.64	0.23	0.23	0.23	0.23	0.23	0.23
Sat Flow, veh/h	229	2532	208	34	3138	99	616	537	227	425	531	415
Grp Volume(v), veh/h	251	0	241	244	0	220	73	0	0	76	0	0
Grp Sat Flow(s),veh/h/ln	1496	0	1473	1717	0	1553	1379	0	0	1372	0	0
Q Serve(g_s), s	0.0	0.0	0.0	0.0	0.0	5.1	0.0	0.0	0.0	0.2	0.0	0.0
Cycle Q Clear(g_c), s	0.0	0.0	0.0	5.0	0.0	5.1	3.1	0.0	0.0	3.4	0.0	0.0
Prop In Lane	0.19		0.14	0.05		0.06	0.51		0.16	0.38		0.30
Lane Grp Cap(c), veh/h	1007	0	941	1141	0	993	378	0	0	371	0	0
V/C Ratio(X)	0.25	0.00	0.26	0.21	0.00	0.22	0.19	0.00	0.00	0.20	0.00	0.00
Avail Cap(c_a), veh/h	1007	0	941	1141	0	993	404	0	0	397	0	0
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.98	0.00	0.98	1.00	0.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	0.0	0.0	0.0	6.4	0.0	6.5	26.5	0.0	0.0	26.6	0.0	0.0
Incr Delay (d2), s/veh	0.6	0.0	0.6	0.4	0.0	0.5	0.2	0.0	0.0	0.3	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	0.0	0.2	1.7	0.0	1.6	1.2	0.0	0.0	1.3	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	0.6	0.0	0.6	6.9	0.0	7.0	26.8	0.0	0.0	26.9	0.0	0.0
LnGrp LOS	Α	Α	Α	Α	Α	Α	С	Α	Α	С	Α	Α
Approach Vol, veh/h		492			464			73			76	
Approach Delay, s/veh		0.6			6.9			26.8			26.9	
Approach LOS		A			A			C			C	
•		2		4	, ,	6		8				
Timer - Assigned Phs												
Phs Duration (G+Y+Rc), s		59.6 * 5.3		25.4		59.6 * 5.3		25.4 6.0				
Change Period (Y+Rc), s		* 53		6.0		* 53		21.0				
Max Green Setting (Gmax), s				21.0								
Max Q Clear Time (g_c+l1), s		2.0		5.4		7.1		5.1				
Green Ext Time (p_c), s		3.6		0.3		3.1		0.3				
Intersection Summary												
HCM 6th Ctrl Delay			6.8									
HCM 6th LOS			Α									
Notes												

<sup>\*</sup> HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection												
Int Delay, s/veh	4.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4î.			ર્ન			<del>(</del> î	
Traffic Vol, veh/h	0	0	0	37	409	11	39	39	0	0	30	44
Future Vol, veh/h	0	0	0	37	409	11	39	39	0	0	30	44
Conflicting Peds, #/hr	30	0	52	52	0	30	29	0	28	28	0	29
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	1	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	0	0	0	5	2	0	0	0	0	0	0	0
Mvmt Flow	0	0	0	42	465	13	44	44	0	0	34	50
Major/Minor				Major2		N	/linor1		N	/linor2		
Conflicting Flow All				52	0	0	415	644	-	-	638	298
Stage 1				-	-	-	52	52	-	-	586	
Stage 2				-	-	-	363	592	-	-	52	-
Critical Hdwy				4.2	_	-	7.5	6.5	-	-	6.5	6.9
Critical Hdwy Stg 1				-	-	-	-	-	-	-	5.5	-
Critical Hdwy Stg 2				-	-	-	6.5	5.5	-	-	-	-
Follow-up Hdwy				2.25	-	-	3.5	4	-	-	4	3.3
Pot Cap-1 Maneuver				1530	-	-	527	394	0	0	397	704
Stage 1				-	-	-	-	-	0	0	500	-
Stage 2				-	-	-	634	497	0	0	-	-
Platoon blocked, %					-	-						
Mov Cap-1 Maneuver				1454	-	-	417	349	-	-	352	684
Mov Cap-2 Maneuver				-	-	-	417	349	-	-	352	-
Stage 1				-	-	-	-	-	-	-	467	-
Stage 2				-	-	-	524	464	-	-	-	-
Approach				WB			NB			SB		
HCM Control Delay, s				0.7			17.3			13.8		
HCM LOS				0.1			17.3			13.0 B		
TIOWI LOO							J			U		
Minor Lane/Major Mvmt	+ N	NBLn1	WBL	WBT	WBR	SRI n1						
Capacity (veh/h)		380	1454	VVD I	- VVDIN	495						
HCM Lane V/C Ratio		0.233		-	-	0.17						
HCM Control Delay (s)		17.3	7.5	0.1	-	13.8						
HCM Lane LOS		17.3 C	7.5 A	Ο.1	-	13.0 B						
HCM 95th %tile Q(veh)		0.9	0.1	- A	-	0.6						
		0.0	U. 1			0.0						

Intersection													
Int Delay, s/veh	4.3												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		414						ĵ.			4		
Traffic Vol, veh/h	27	392	26	0	0	0	0	38	38	38	50	0	
Future Vol, veh/h	27	392	26	0	0	0	0	38	38	38	50	0	
Conflicting Peds, #/hr	15	0	11	11	0	15	32	0	14	14	0	32	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	<u> </u>	-	None	
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85	
Heavy Vehicles, %	0	1	0	0	0	0	0	0	0	0	0	0	
Mvmt Flow	32	461	31	0	0	0	0	45	45	45	59	0	
Major/Minor N	/lajor1					<b>N</b>	Minor1		ľ	Minor2			
Conflicting Flow All	15	0	0				-	567	271	346	582	-	
Stage 1	-	-	-				-	552	-	15	15	-	
Stage 2	-	-	-				-	15	-	331	567	-	
Critical Hdwy	4.1	-	-				-	6.5	6.9	7.5	6.5	-	
Critical Hdwy Stg 1	-	-	-				-	5.5	-	-	-	-	
Critical Hdwy Stg 2	-	-	-				-	-	-	6.5	5.5	-	
Follow-up Hdwy	2.2	-	-				-	4	3.3	3.5	4	-	
Pot Cap-1 Maneuver	1616	-	-				0	436	733	589	427	0	
Stage 1	-	-	-				0	518	-	-	-	0	
Stage 2	-	-	-				0	-	-	662	510	0	
Platoon blocked, %		-	-										
Mov Cap-1 Maneuver	1593	-	-				-	414	725	489	405	-	
Mov Cap-2 Maneuver	-	-	-				-	414	-	489	405	-	
Stage 1	-	-	-				-	498	-	-	-	-	
Stage 2	-	-	-				-	-	-	550	491	-	
Approach	EB						NB			SB			
HCM Control Delay, s	0.5						13.2			15.8			
HCM LOS							В			С			
Minor Lane/Major Mvmt	t <u></u> N	NBLn1	EBL	EBT	EBR	SBLn1							
Capacity (veh/h)		527	1593	-	-								
HCM Lane V/C Ratio		0.17	0.02	-	-	0.237							
HCM Control Delay (s)		13.2	7.3	0.1	-	15.8							
HCM Lane LOS		В	Α	Α	-	С							
HCM 95th %tile Q(veh)		0.6	0.1	-	-	0.9							

Approach	EB	WB	NB	SB
Crosswalk Length (ft)	65.0	65.0	40.0	40.0
Crosswalk Width (ft)	12.0	12.0	12.0	12.0
Total Number of Lanes Crossed	4	4	2	2
Number of Right-Turn Islands	0	0	0	0
Type of Control	Pretimed P	retimed P	retimed P	retimed
Corresponding Signal Phase	4	8	2	6
Effective Walk Time (s)	11.0	11.0	11.0	11.0
Right Corner Size A (ft)	9.0	9.0	9.0	9.0
Right Corner Size B (ft)	9.0	9.0	9.0	9.0
Right Corner Curb Radius (ft)	0.0	0.0	0.0	0.0
Right Corner Total Area (sq.ft)	81.00	81.00	81.00	81.00
Ped. Left-Right Flow Rate (p/h)	16	23	46	36
Ped. Right-Left Flow Rate (p/h)	16	23	46	37
Ped. R. Sidewalk Flow Rate (p/h)	0	0	0	0
Veh. Perm. L. Flow in Walk (v/h)	25	18	17	17
Veh. Perm. R. Flow in Walk (v/h)	31	22	16	18
Veh. RTOR Flow in Walk (v/h)	3	2	2	2
85th percentile speed (mph)	30	30	30	30
Right Corner Area per Ped (sq.ft)	570.8	595.7	511.2	677.1
Right Corner Quality of Service	Α	Α	Α	Α
Ped. Circulation Area (sq.ft)	616.5	436.8	193.0	242.9
Crosswalk Circulation Code	Α	Α	Α	Α
Pedestrian Delay (s/p)	27.3	27.3	27.3	27.3
Pedestrian Compliance Code	Fair	Fair	Fair	Fair
Pedestrian Crosswalk Score	2.36	2.35	1.80	1.80
Pedestrian Crosswalk LOS	В	В	В	В

Approach		
Approach Direction	EB	
Median Present?	No	
Approach Delay(s)	6.9	
Level of Service	В	
Crosswalk		
Length (ft)	40	
Lanes Crossed	2	
Veh Vol Crossed	257	
Ped Vol Crossed	22	
Yield Rate(%)	50	
Ped Platooning	No	
<b>,</b>		
Critical Headway (s)	14.43	
Prob of Delayed X-ing	0.64	
Prob of Blocked Lane	0.40	
Delay for adq Gap	16.80	
Avg Ped Delay (s)	6.95	
, , ,		
Approach		
Approach Direction	WB	
Median Present?	No	
Approach Delay(s)	6.9	
Level of Service	В	
Crosswalk		
Length (ft)	40	
Lanes Crossed	2	
Veh Vol Crossed		
Ped Vol Crossed	41	
Yield Rate(%)	50	
Ped Platooning	No	
Critical Headway (s)	14.43	
Prob of Delayed X-ing	0.64	
Prob of Blocked Lane	0.40	
Delay for adq Gap	16.80	
Avg Ped Delay (s)	6.95	
3		

A	
Approach	ED.
Approach Direction	EB
Median Present?	No
Approach Delay(s)	17.0
Level of Service	С
Crosswalk	
Length (ft)	50
Lanes Crossed	2
Veh Vol Crossed	352
Ped Vol Crossed	16
Yield Rate(%)	50
Ped Platooning	No
r ou r lateoning	110
Critical Headway (s)	17.29
Prob of Delayed X-ing	0.82
Prob of Blocked Lane	0.57
Delay for adq Gap	34.24
Avg Ped Delay (s)	17.00
rivg r ou boldy (5)	17.00
Approach	
Approach Direction	WB
Median Present?	No
Approach Delay(s)	9.0
Level of Service	В
Crosswalk	
	36
Length (ft) Lanes Crossed	2
Veh Vol Crossed	352 26
Ped Vol Crossed	
Yield Rate(%)	50
Ped Platooning	No
Critical Handway (-)	40.00
Critical Headway (s)	13.29
Prob of Delayed X-ing	0.73
Prob of Blocked Lane	0.48
Delay for adq Gap	19.22
Avg Ped Delay (s)	9.02

Approach	EB	WB	NB	SB
Crosswalk Length (ft)	60.0	60.0	40.0	40.0
Crosswalk Width (ft)	12.0	12.0	12.0	12.0
Total Number of Lanes Crossed	4	4	2	2
Number of Right-Turn Islands	0	0	0	0
Type of Control	Pretimed F	Pretimed P	retimed P	retimed
Corresponding Signal Phase	4	8	2	6
Effective Walk Time (s)	11.0	11.0	11.0	11.0
Right Corner Size A (ft)	9.0	9.0	9.0	9.0
Right Corner Size B (ft)	9.0	9.0	9.0	9.0
Right Corner Curb Radius (ft)	0.0	0.0	0.0	0.0
Right Corner Total Area (sq.ft)	81.00	81.00	81.00	81.00
Ped. Left-Right Flow Rate (p/h)	46	8	32	59
Ped. Right-Left Flow Rate (p/h)	47	9	32	59
Ped. R. Sidewalk Flow Rate (p/h)	0	0	0	0
Veh. Perm. L. Flow in Walk (v/h)	38	14	18	18
Veh. Perm. R. Flow in Walk (v/h)	29	23	18	18
Veh. RTOR Flow in Walk (v/h)	3	2	2	2
85th percentile speed (mph)	30	30	30	30
Right Corner Area per Ped (sq.ft)	447.2	522.8	883.1	328.5
Right Corner Quality of Service	Α	Α	Α	Α
Ped. Circulation Area (sq.ft)	203.5	1169.1	276.9	149.0
Crosswalk Circulation Code	Α	Α	Α	Α
Pedestrian Delay (s/p)	27.3	27.3	27.3	27.3
Pedestrian Compliance Code	Fair	Fair	Fair	Fair
Pedestrian Crosswalk Score	2.40	2.36	1.81	1.82
Pedestrian Crosswalk LOS	В	В	В	В

Approach	
Approach Direction	EB
Median Present?	No
Approach Delay(s)	6.7
Level of Service	В
Crosswalk	
Length (ft)	40
Lanes Crossed	2
Veh Vol Crossed	252
Ped Vol Crossed	25
Yield Rate(%)	50
Ped Platooning	No
Critical Headway (s)	14.43
Prob of Delayed X-ing	0.64
Prob of Blocked Lane	0.40
Delay for adq Gap	16.53
Avg Ped Delay (s)	6.68
Approach	
Approach Direction	WB
Median Present?	No
Approach Delay(s)	6.7
Level of Service	В
Crosswalk	
Length (ft)	40
Lanes Crossed	2
Veh Vol Crossed	252
Ped Vol Crossed	22
Yield Rate(%)	50
Ped Platooning	No
O-iti	44.40
Critical Headway (s)	14.43
Prob of Delayed X-ing	0.64
Prob of Blocked Lane	0.40
Delay for adq Gap	16.53 6.68
Avg Ped Delay (s)	0.00

Approach		
Approach Direction	EB	
Median Present?	No	
Approach Delay(s)	10.9	
Level of Service	С	
Crosswalk		
Length (ft)	40	
Lanes Crossed	2	
Veh Vol Crossed	354	
Ped Vol Crossed	21	
Yield Rate(%)	50	
Ped Platooning	No	
r ca r latooning	110	
Critical Headway (s)	14.43	
Prob of Delayed X-ing	0.76	
Prob of Blocked Lane	0.51	
Delay for adq Gap	22.99	
Avg Ped Delay (s)	10.93	
Avg r eu Delay (5)	10.33	
Approach		
Approach Direction	WB	
Median Present?	No	
Approach Delay(s)	10.9	
Level of Service	С	
Crosswalk		
Length (ft)	40	
Lanes Crossed	2	
Veh Vol Crossed	354	
Ped Vol Crossed	6	
Yield Rate(%)	50	
Ped Platooning	No	
Critical Headway (s)	14.43	
Prob of Delayed X-ing	0.76	
Prob of Blocked Lane	0.51	
Delay for adq Gap	22.99	
Avg Ped Delay (s)	10.93	
g r ou bold, (o)	10.00	

Approach	EB	WB	NB	SB
Crosswalk Length (ft)	65.0	65.0	40.0	40.0
Crosswalk Width (ft)	12.0	12.0	12.0	12.0
Total Number of Lanes Crossed	4	4	2	2
Number of Right-Turn Islands	0	0	0	0
Type of Control	Pretimed P	retimed P	retimed P	retimed
Corresponding Signal Phase	4	8	2	6
Effective Walk Time (s)	11.0	11.0	11.0	11.0
Right Corner Size A (ft)	9.0	9.0	9.0	9.0
Right Corner Size B (ft)	9.0	9.0	9.0	9.0
Right Corner Curb Radius (ft)	0.0	0.0	0.0	0.0
Right Corner Total Area (sq.ft)	81.00	81.00	81.00	81.00
Ped. Left-Right Flow Rate (p/h)	48	73	213	236
Ped. Right-Left Flow Rate (p/h)	49	73	213	235
Ped. R. Sidewalk Flow Rate (p/h)	0	0	0	0
Veh. Perm. L. Flow in Walk (v/h)	24	22	29	39
Veh. Perm. R. Flow in Walk (v/h)	53	42	13	28
Veh. RTOR Flow in Walk (v/h)	5	4	2	3
85th percentile speed (mph)	30	30	30	30
Right Corner Area per Ped (sq.ft)	119.2	98.0	107.3	108.3
Right Corner Quality of Service	Α	Α	Α	Α
Ped. Circulation Area (sq.ft)	171.0	115.2	33.7	28.4
Crosswalk Circulation Code	Α	Α	С	С
Pedestrian Delay (s/p)	32.2	32.2	32.2	32.2
Pedestrian Compliance Code	Poor	Poor	Poor	Poor
Pedestrian Crosswalk Score	2.43	2.42	1.86	1.88
Pedestrian Crosswalk LOS	В	В	В	В

Approach	
Approach Direction	EB
Median Present?	No
Approach Delay(s)	12.9
Level of Service	С
Crosswalk	
	40
Length (ft) Lanes Crossed	2
Veh Vol Crossed	398
Ped Vol Crossed	398
	50
Yield Rate(%)	
Ped Platooning	No
0.26 - 111 - 1 (-)	44.40
Critical Headway (s)	14.43
Prob of Delayed X-ing	0.80
Prob of Blocked Lane	0.55
Delay for adq Gap	26.48
Avg Ped Delay (s)	12.93
Approach	
Approach Direction	WB
Median Present?	No
Approach Delay(s)	12.9
Level of Service	C
	Ŭ
Crosswalk	
Length (ft)	40
Lanes Crossed	2
Veh Vol Crossed	398
Ped Vol Crossed	83
Yield Rate(%)	50
Ped Platooning	No
Critical Headway (s)	14.43
Prob of Delayed X-ing	0.80
Prob of Blocked Lane	0.55
Delay for adq Gap	26.48
Avg Ped Delay (s)	12.93

Approach	
Approach Direction	EB
Median Present?	No
Approach Delay(s)	22.3
Level of Service	D
Crosswalk	
Length (ft)	50
Lanes Crossed	2
Veh Vol Crossed	451
Ped Vol Crossed	38
Yield Rate(%)	50
Ped Platooning	No
r cu r latoorning	140
Critical Headway (s)	17.29
Prob of Delayed X-ing	0.89
Prob of Blocked Lane	0.66
Delay for adq Gap	50.07
Avg Ped Delay (s)	22.32
Avg r cu belay (3)	22.02
Approach	
Approach Direction	WB
Median Present?	No
Approach Delay(s)	12.7
Level of Service	С
Crosswalk	
Length (ft)	36
Lanes Crossed	2
Veh Vol Crossed	451
Ped Vol Crossed	35
Yield Rate(%)	50
Ped Platooning	No
9	
Critical Headway (s)	13.29
Prob of Delayed X-ing	0.81
Prob of Blocked Lane	0.56
Delay for adq Gap	25.78
Avg Ped Delay (s)	12.74
J	

Crosswalk Length (ft)         60.0         60.0         40.0         40.0           Crosswalk Width (ft)         12.0         12.0         12.0         12.0           Total Number of Lanes Crossed         4         4         2         2           Number of Right-Turn Islands         0         0         0         0           Type of Control         Pretimed Pre					
Crosswalk Width (ft)         12.0         12.0         12.0         12.0           Total Number of Lanes Crossed         4         4         2         2           Number of Right-Turn Islands         0         0         0         0           Type of Control         Pretimed Preti	Approach	EB	WB	NB	SB
Total Number of Lanes Crossed         4         4         2         2           Number of Right-Turn Islands         0         0         0         0           Type of Control         Pretimed Pretimed Pretimed Pretimed         Pretimed Pretimed Pretimed Pretimed         Pretimed Pretimed Pretimed Pretimed           Corresponding Signal Phase         4         8         2         6           Effective Walk Time (s)         11.0 <td>Crosswalk Length (ft)</td> <td>60.0</td> <td>60.0</td> <td>40.0</td> <td>40.0</td>	Crosswalk Length (ft)	60.0	60.0	40.0	40.0
Number of Right-Turn Islands         0         0         0         0           Type of Control         Pretimed Pretimed Pretimed Pretimed Pretimed           Corresponding Signal Phase         4         8         2         6           Effective Walk Time (s)         11.0         10.0         10.0         10.0         10.0	Crosswalk Width (ft)	12.0	12.0	12.0	12.0
Type of Control         Pretimed Pretimed Pretimed Pretimed           Corresponding Signal Phase         4         8         2         6           Effective Walk Time (s)         11.0         11.0         11.0         11.0           Right Corner Size A (ft)         9.0         9.0         9.0         9.0           Right Corner Size B (ft)         9.0         9.0         9.0         9.0           Right Corner Curb Radius (ft)         0.0         0.0         0.0         0.0           Right Corner Total Area (sq.ft)         81.00         81.00         81.00         81.00           Ped. Left-Right Flow Rate (p/h)         127         44         185         251           Ped. Right-Left Flow Rate (p/h)         127         44         185         251           Ped. R. Sidewalk Flow Rate (p/h)         0         0         0         0           Veh. Perm. L. Flow in Walk (v/h)         50         16         23         35           Veh. Perm. R. Flow in Walk (v/h)         24         30         12         23           85th percentile speed (mph)         30         30         30         30           Right Corner Area per Ped (sq.ft)         96.7         103.4         139.0         76.3	Total Number of Lanes Crossed	4	4	2	2
Corresponding Signal Phase         4         8         2         6           Effective Walk Time (s)         11.0         11.0         11.0         11.0           Right Corner Size A (ft)         9.0         9.0         9.0         9.0           Right Corner Size B (ft)         9.0         9.0         9.0         9.0           Right Corner Curb Radius (ft)         0.0         0.0         0.0         0.0           Right Corner Total Area (sq.ft)         81.00         81.00         81.00         81.00           Ped. Left-Right Flow Rate (p/h)         127         44         185         251           Ped. Right-Left Flow Rate (p/h)         127         44         185         251           Ped. R. Sidewalk Flow Rate (p/h)         0         0         0         0           Veh. Perm. L. Flow in Walk (v/h)         50         16         23         35           Veh. Perm. R. Flow in Walk (v/h)         24         30         12         23           85th percentile speed (mph)         30         30         30         30           85th percentile speed (mph)         30         30         30         30           Right Corner Area per Ped (sq.ft)         96.7         103.4         139.0 </td <td>Number of Right-Turn Islands</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td>	Number of Right-Turn Islands	0	0	0	0
Effective Walk Time (s)       11.0	Type of Control	Pretimed P	retimed P	retimed P	retimed
Right Corner Size A (ft)       9.0       9.0       9.0       9.0         Right Corner Size B (ft)       9.0       9.0       9.0       9.0         Right Corner Curb Radius (ft)       0.0       0.0       0.0       0.0         Right Corner Total Area (sq.ft)       81.00       81.00       81.00       81.00         Ped. Left-Right Flow Rate (p/h)       127       44       185       251         Ped. Right-Left Flow Rate (p/h)       0       0       0       0         Veh. Perm. L. Flow in Walk (v/h)       50       16       23       35         Veh. Perm. R. Flow in Walk (v/h)       24       30       12       23         Veh. RTOR Flow in Walk (v/h)       2       3       2       2         85th percentile speed (mph)       30       30       30       30         Right Corner Area per Ped (sq.ft)       96.7       103.4       139.0       76.3         Right Corner Quality of Service       A       A       A         Ped. Circulation Area (sq.ft)       62.8       193.5       39.8       27.2         Crosswalk Circulation Code       A       A       C       C         Pedestrian Delay (s/p)       32.2       32.2       32.2 <t< td=""><td>Corresponding Signal Phase</td><td>4</td><td>8</td><td>2</td><td>6</td></t<>	Corresponding Signal Phase	4	8	2	6
Right Corner Size B (ft)       9.0       9.0       9.0       9.0         Right Corner Curb Radius (ft)       0.0       0.0       0.0       0.0         Right Corner Total Area (sq.ft)       81.00       81.00       81.00       81.00         Ped. Left-Right Flow Rate (p/h)       127       44       185       251         Ped. Right-Left Flow Rate (p/h)       0       0       0       0         Veh. Right Flow Rate (p/h)       0       0       0       0         Veh. Perm. L. Flow in Walk (v/h)       50       16       23       35         Veh. Perm. R. Flow in Walk (v/h)       24       30       12       23         Veh. RTOR Flow in Walk (v/h)       2       3       2       2         85th percentile speed (mph)       30       30       30       30         Right Corner Area per Ped (sq.ft)       96.7       103.4       139.0       76.3         Right Corner Quality of Service       A       A       A       A         Ped. Circulation Area (sq.ft)       62.8       193.5       39.8       27.2         Crosswalk Circulation Code       A       A       C       C         Pedestrian Delay (s/p)       32.2       32.2       32.2 <td>Effective Walk Time (s)</td> <td>11.0</td> <td>11.0</td> <td>11.0</td> <td>11.0</td>	Effective Walk Time (s)	11.0	11.0	11.0	11.0
Right Corner Curb Radius (ft)         0.0         0.0         0.0         0.0           Right Corner Total Area (sq.ft)         81.00         81.00         81.00         81.00           Ped. Left-Right Flow Rate (p/h)         127         44         185         251           Ped. Right-Left Flow Rate (p/h)         0         0         0         0           Veh. Right-Left Flow Rate (p/h)         0         0         0         0           Veh. Right Corner L. Flow in Walk (v/h)         50         16         23         35           Veh. Perm. R. Flow in Walk (v/h)         24         30         12         23           Veh. RTOR Flow in Walk (v/h)         2         3         2         2           85th percentile speed (mph)         30         30         30         30           Right Corner Area per Ped (sq.ft)         96.7         103.4         139.0         76.3           Right Corner Quality of Service         A         A         A         A           Ped. Circulation Area (sq.ft)         62.8         193.5         39.8         27.2           Crosswalk Circulation Code         A         A         C         C           Pedestrian Delay (s/p)         32.2         32.2         32	Right Corner Size A (ft)	9.0	9.0	9.0	9.0
Right Corner Total Area (sq.ft)       81.00       81.00       81.00       81.00         Ped. Left-Right Flow Rate (p/h)       127       44       185       251         Ped. Right-Left Flow Rate (p/h)       127       44       185       251         Ped. R. Sidewalk Flow Rate (p/h)       0       0       0       0         Veh. Perm. L. Flow in Walk (v/h)       50       16       23       35         Veh. Perm. R. Flow in Walk (v/h)       24       30       12       23         Veh. RTOR Flow in Walk (v/h)       2       3       2       2         85th percentile speed (mph)       30       30       30       30         85th percentile speed (sq.ft)       96.7       103.4       139.0       76.3         Right Corner Area per Ped (sq.ft)       96.7       103.4       139.0       76.3         Right Corner Quality of Service       A       A       A       A         Ped. Circulation Area (sq.ft)       62.8       193.5       39.8       27.2         Crosswalk Circulation Code       A       A       C       C         Pedestrian Delay (s/p)       32.2       32.2       32.2       32.2       32.2       32.2         Pedestrian Crosswalk Sco	Right Corner Size B (ft)	9.0	9.0	9.0	9.0
Ped. Left-Right Flow Rate (p/h)         127         44         185         251           Ped. Right-Left Flow Rate (p/h)         127         44         185         251           Ped. R. Sidewalk Flow Rate (p/h)         0         0         0         0           Veh. Perm. L. Flow in Walk (v/h)         50         16         23         35           Veh. Perm. R. Flow in Walk (v/h)         24         30         12         23           Veh. RTOR Flow in Walk (v/h)         2         3         2         2           85th percentile speed (mph)         30         30         30         30           Right Corner Area per Ped (sq.ft)         96.7         103.4         139.0         76.3           Right Corner Quality of Service         A         A         A         A           Ped. Circulation Area (sq.ft)         62.8         193.5         39.8         27.2           Crosswalk Circulation Code         A         A         C         C           Pedestrian Delay (s/p)         32.2         32.2         32.2         32.2           Pedestrian Crosswalk Score         2.49         2.43         1.82         1.87	Right Corner Curb Radius (ft)	0.0	0.0	0.0	0.0
Ped. Right-Left Flow Rate (p/h)         127         44         185         251           Ped. R. Sidewalk Flow Rate (p/h)         0         0         0         0           Veh. Perm. L. Flow in Walk (v/h)         50         16         23         35           Veh. Perm. R. Flow in Walk (v/h)         24         30         12         23           Veh. RTOR Flow in Walk (v/h)         2         3         2         2           85th percentile speed (mph)         30         30         30         30           Right Corner Area per Ped (sq.ft)         96.7         103.4         139.0         76.3           Right Corner Quality of Service         A         A         A         A           Ped. Circulation Area (sq.ft)         62.8         193.5         39.8         27.2           Crosswalk Circulation Code         A         A         C         C           Pedestrian Delay (s/p)         32.2         32.2         32.2         32.2           Pedestrian Compliance Code         Poor         Poor         Poor         Poor           Pedestrian Crosswalk Score         2.49         2.43         1.82         1.87	Right Corner Total Area (sq.ft)	81.00	81.00	81.00	81.00
Ped. R. Sidewalk Flow Rate (p/h)         0         0         0         0           Veh. Perm. L. Flow in Walk (v/h)         50         16         23         35           Veh. Perm. R. Flow in Walk (v/h)         24         30         12         23           Veh. RTOR Flow in Walk (v/h)         2         3         2         2           85th percentile speed (mph)         30         30         30         30           Right Corner Area per Ped (sq.ft)         96.7         103.4         139.0         76.3           Right Corner Quality of Service         A         A         A         A           Ped. Circulation Area (sq.ft)         62.8         193.5         39.8         27.2           Crosswalk Circulation Code         A         A         C         C           Pedestrian Delay (s/p)         32.2         32.2         32.2         32.2           Pedestrian Compliance Code         Poor         Poor         Poor         Poor           Pedestrian Crosswalk Score         2.49         2.43         1.82         1.87	Ped. Left-Right Flow Rate (p/h)	127	44	185	251
Veh. Perm. L. Flow in Walk (v/h)         50         16         23         35           Veh. Perm. R. Flow in Walk (v/h)         24         30         12         23           Veh. RTOR Flow in Walk (v/h)         2         3         2         2           85th percentile speed (mph)         30         30         30         30           Right Corner Area per Ped (sq.ft)         96.7         103.4         139.0         76.3           Right Corner Quality of Service         A         A         A         A           Ped. Circulation Area (sq.ft)         62.8         193.5         39.8         27.2           Crosswalk Circulation Code         A         A         C         C           Pedestrian Delay (s/p)         32.2         32.2         32.2         32.2           Pedestrian Compliance Code         Poor         Poor         Poor         Poor           Pedestrian Crosswalk Score         2.49         2.43         1.82         1.87	Ped. Right-Left Flow Rate (p/h)	127	44	185	251
Veh. Perm. R. Flow in Walk (v/h)         24         30         12         23           Veh. RTOR Flow in Walk (v/h)         2         3         2         2           85th percentile speed (mph)         30         30         30         30           Right Corner Area per Ped (sq.ft)         96.7         103.4         139.0         76.3           Right Corner Quality of Service         A         A         A         A           Ped. Circulation Area (sq.ft)         62.8         193.5         39.8         27.2           Crosswalk Circulation Code         A         A         C         C           Pedestrian Delay (s/p)         32.2         32.2         32.2         32.2           Pedestrian Compliance Code         Poor         Poor         Poor         Poor           Pedestrian Crosswalk Score         2.49         2.43         1.82         1.87	Ped. R. Sidewalk Flow Rate (p/h)	0	0	0	0
Veh. RTOR Flow in Walk (v/h)         2         3         2         2           85th percentile speed (mph)         30         30         30         30           Right Corner Area per Ped (sq.ft)         96.7         103.4         139.0         76.3           Right Corner Quality of Service         A         A         A         A           Ped. Circulation Area (sq.ft)         62.8         193.5         39.8         27.2           Crosswalk Circulation Code         A         A         C         C           Pedestrian Delay (s/p)         32.2         32.2         32.2         32.2           Pedestrian Compliance Code         Poor         Poor         Poor         Poor           Pedestrian Crosswalk Score         2.49         2.43         1.82         1.87	Veh. Perm. L. Flow in Walk (v/h)	50	16	23	35
85th percentile speed (mph)  Right Corner Area per Ped (sq.ft)  Right Corner Quality of Service  A  A  A  Ped. Circulation Area (sq.ft)  Crosswalk Circulation Code  Pedestrian Delay (s/p)  Pedestrian Compliance Code  Pedestrian Crosswalk Score  30  30  30  30  30  30  30  30  30  3	Veh. Perm. R. Flow in Walk (v/h)	24	30	12	23
Right Corner Area per Ped (sq.ft) 96.7 103.4 139.0 76.3 Right Corner Quality of Service A A A A A A A A A A A A A A A A A A A	Veh. RTOR Flow in Walk (v/h)	2	3	2	2
Right Corner Quality of Service A A A A A A A A A A A A A A A A A A A	85th percentile speed (mph)	30	30	30	30
Ped. Circulation Area (sq.ft)62.8193.539.827.2Crosswalk Circulation CodeAACCPedestrian Delay (s/p)32.232.232.232.2Pedestrian Compliance CodePoorPoorPoorPoorPedestrian Crosswalk Score2.492.431.821.87	Right Corner Area per Ped (sq.ft)	96.7	103.4	139.0	76.3
Crosswalk Circulation Code  Pedestrian Delay (s/p)  Pedestrian Compliance Code  Poor  Pedestrian Crosswalk Score  A  A  C  C  C  C  C  C  C  C  C  C  C	Right Corner Quality of Service	Α	Α	Α	Α
Pedestrian Delay (s/p)32.232.232.232.2Pedestrian Compliance CodePoorPoorPoorPoorPedestrian Crosswalk Score2.492.431.821.87	Ped. Circulation Area (sq.ft)	62.8	193.5	39.8	27.2
Pedestrian Compliance CodePoorPoorPoorPoorPedestrian Crosswalk Score2.492.431.821.87	Crosswalk Circulation Code	Α	Α	С	С
Pedestrian Crosswalk Score 2.49 2.43 1.82 1.87	Pedestrian Delay (s/p)	32.2	32.2	32.2	32.2
	Pedestrian Compliance Code	Poor	Poor	Poor	Poor
Pedestrian Crosswalk LOS B B B	Pedestrian Crosswalk Score	2.49	2.43	1.82	1.87
	Pedestrian Crosswalk LOS	В	В	В	В

Approach		
Approach Direction	EB	
Median Present?	No	
Approach Delay(s)	12.1	
Level of Service	C	
Crosswalk		
Length (ft)	40	
Lanes Crossed	2	
Veh Vol Crossed	380	
Ped Vol Crossed	58	
Yield Rate(%)	50	
Ped Platooning	No	
r ed r latooning	INO	
Critical Headway (s)	14.43	
Prob of Delayed X-ing	0.78	
Prob of Blocked Lane	0.53	
Delay for adq Gap	24.99	
Avg Ped Delay (s)	12.06	
ring rod Boldy (o)	12.00	
A I		
Approach		
Approach Direction	WB	
Median Present?	No	
Approach Delay(s)	12.1	
Level of Service	С	
Crosswalk		
Length (ft)	40	
Lanes Crossed	2	
Veh Vol Crossed	380	
Ped Vol Crossed	29	
Yield Rate(%)	50	
Ped Platooning	No	
Critical Headway (s)	14.43	
Prob of Delayed X-ing	0.78	
Prob of Blocked Lane	0.53	
Delay for adq Gap	24.99	
Avg Ped Delay (s)	12.06	

Crosswalk Length (ft)         65.0         65.0         40.0         40.0           Crosswalk Width (ft)         12.0         12.0         12.0         12.0           Total Number of Lanes Crossed         4         4         2         2           Number of Right-Turn Islands         0         0         0         0           Type of Control         Pretimed Pre					
Crosswalk Width (ft)         12.0         12.0         12.0         12.0           Total Number of Lanes Crossed         4         4         2         2           Number of Right-Turn Islands         0         0         0         0           Type of Control         Pretimed Preti	Approach	EB	WB	NB	SB
Total Number of Lanes Crossed         4         4         2         2           Number of Right-Turn Islands         0         0         0         0           Type of Control         Pretimed Pretimed Pretimed Pretimed           Corresponding Signal Phase         4         8         2         6           Effective Walk Time (s)         11.0         10.0         10.	Crosswalk Length (ft)	65.0	65.0	40.0	40.0
Number of Right-Turn Islands         0         0         0         0           Type of Control         Pretimed Pretimed Pretimed Pretimed Pretimed           Corresponding Signal Phase         4         8         2         6           Effective Walk Time (s)         11.0         12.0         12.0         12.0         11.0	Crosswalk Width (ft)	12.0	12.0	12.0	12.0
Type of Control         Pretimed Pretimed Pretimed Pretimed Corresponding Signal Phase         4         8         2         6           Effective Walk Time (s)         11.0         12.0         12.0         12.0         12.0         12.0         12.0         12.0         12.0         12.0         12.0         12.0         12.0         12.0	Total Number of Lanes Crossed	4	4	2	2
Corresponding Signal Phase         4         8         2         6           Effective Walk Time (s)         11.0	Number of Right-Turn Islands	0	0	0	0
Effective Walk Time (s)         11.0         10.0         10.0         0.0         0.0         0.0         0.0         81.00         9.0         9.0         0         0	Type of Control	Pretimed P	retimed P	retimed P	retimed
Right Corner Size A (ft)       9.0       9.0       9.0       9.0         Right Corner Size B (ft)       9.0       9.0       9.0       9.0         Right Corner Curb Radius (ft)       0.0       0.0       0.0       0.0         Right Corner Total Area (sq.ft)       81.00       81.00       81.00       81.00         Ped. Left-Right Flow Rate (p/h)       19       43       111       161         Ped. Right-Left Flow Rate (p/h)       20       43       111       161         Ped. R. Sidewalk Flow Rate (p/h)       0       0       0       0         Veh. Perm. L. Flow in Walk (v/h)       24       11       31       23         Veh. Perm. R. Flow in Walk (v/h)       29       15       14       23         Veh. RTOR Flow in Walk (v/h)       3       2       2       2         85th percentile speed (mph)       30       30       30       30         Right Corner Area per Ped (sq.ft)       259.1       158.5       216.6       181.9         Right Corner Quality of Service       A       A       A       A         Ped. Circulation Area (sq.ft)       443.5       207.7       66.4       44.9         Crosswalk Circulation Code       A       A	Corresponding Signal Phase	4	8	2	6
Right Corner Size B (ft)       9.0       9.0       9.0       9.0         Right Corner Curb Radius (ft)       0.0       0.0       0.0       0.0         Right Corner Total Area (sq.ft)       81.00       81.00       81.00       81.00         Ped. Left-Right Flow Rate (p/h)       19       43       111       161         Ped. Right-Left Flow Rate (p/h)       20       43       111       161         Ped. R. Sidewalk Flow Rate (p/h)       0       0       0       0         Veh. Perm. L. Flow in Walk (v/h)       24       11       31       23         Veh. Perm. R. Flow in Walk (v/h)       29       15       14       23         Veh. RTOR Flow in Walk (v/h)       3       2       2       2         85th percentile speed (mph)       30       30       30       30         Right Corner Area per Ped (sq.ft)       259.1       158.5       216.6       181.9         Right Corner Quality of Service       A       A       A       A         Ped. Circulation Area (sq.ft)       443.5       207.7       66.4       44.9         Crosswalk Circulation Code       A       A       A       B	Effective Walk Time (s)	11.0	11.0	11.0	11.0
Right Corner Curb Radius (ft)       0.0       0.0       0.0       0.0         Right Corner Total Area (sq.ft)       81.00       81.00       81.00       81.00         Ped. Left-Right Flow Rate (p/h)       19       43       111       161         Ped. Right-Left Flow Rate (p/h)       20       43       111       161         Ped. R. Sidewalk Flow Rate (p/h)       0       0       0       0         Veh. Perm. L. Flow in Walk (v/h)       24       11       31       23         Veh. Perm. R. Flow in Walk (v/h)       29       15       14       23         Veh. RTOR Flow in Walk (v/h)       3       2       2       2         85th percentile speed (mph)       30       30       30       30         Right Corner Area per Ped (sq.ft)       259.1       158.5       216.6       181.9         Right Corner Quality of Service       A       A       A       A         Ped. Circulation Area (sq.ft)       443.5       207.7       66.4       44.9         Crosswalk Circulation Code       A       A       A       B	Right Corner Size A (ft)	9.0	9.0	9.0	9.0
Right Corner Total Area (sq.ft)       81.00	Right Corner Size B (ft)	9.0	9.0	9.0	9.0
Ped. Left-Right Flow Rate (p/h)       19       43       111       161         Ped. Right-Left Flow Rate (p/h)       20       43       111       161         Ped. R. Sidewalk Flow Rate (p/h)       0       0       0       0         Veh. Perm. L. Flow in Walk (v/h)       24       11       31       23         Veh. Perm. R. Flow in Walk (v/h)       29       15       14       23         Veh. RTOR Flow in Walk (v/h)       3       2       2       2         85th percentile speed (mph)       30       30       30       30         Right Corner Area per Ped (sq.ft)       259.1       158.5       216.6       181.9         Right Corner Quality of Service       A       A       A       A         Ped. Circulation Area (sq.ft)       443.5       207.7       66.4       44.9         Crosswalk Circulation Code       A       A       A       B	Right Corner Curb Radius (ft)	0.0	0.0	0.0	0.0
Ped. Right-Left Flow Rate (p/h)       20       43       111       161         Ped. R. Sidewalk Flow Rate (p/h)       0       0       0       0         Veh. Perm. L. Flow in Walk (v/h)       24       11       31       23         Veh. Perm. R. Flow in Walk (v/h)       29       15       14       23         Veh. RTOR Flow in Walk (v/h)       3       2       2       2         85th percentile speed (mph)       30       30       30       30         Right Corner Area per Ped (sq.ft)       259.1       158.5       216.6       181.9         Right Corner Quality of Service       A       A       A       A         Ped. Circulation Area (sq.ft)       443.5       207.7       66.4       44.9         Crosswalk Circulation Code       A       A       A       B	Right Corner Total Area (sq.ft)	81.00	81.00	81.00	81.00
Ped. R. Sidewalk Flow Rate (p/h)       0       0       0         Veh. Perm. L. Flow in Walk (v/h)       24       11       31       23         Veh. Perm. R. Flow in Walk (v/h)       29       15       14       23         Veh. RTOR Flow in Walk (v/h)       3       2       2       2         85th percentile speed (mph)       30       30       30       30         Right Corner Area per Ped (sq.ft)       259.1       158.5       216.6       181.9         Right Corner Quality of Service       A       A       A       A         Ped. Circulation Area (sq.ft)       443.5       207.7       66.4       44.9         Crosswalk Circulation Code       A       A       A       B	Ped. Left-Right Flow Rate (p/h)	19	43	111	161
Veh. Perm. L. Flow in Walk (v/h)       24       11       31       23         Veh. Perm. R. Flow in Walk (v/h)       29       15       14       23         Veh. RTOR Flow in Walk (v/h)       3       2       2       2         85th percentile speed (mph)       30       30       30       30         Right Corner Area per Ped (sq.ft)       259.1       158.5       216.6       181.9         Right Corner Quality of Service       A       A       A       A         Ped. Circulation Area (sq.ft)       443.5       207.7       66.4       44.9         Crosswalk Circulation Code       A       A       A       B	Ped. Right-Left Flow Rate (p/h)	20	43	111	161
Veh. Perm. R. Flow in Walk (v/h)       29       15       14       23         Veh. RTOR Flow in Walk (v/h)       3       2       2       2         85th percentile speed (mph)       30       30       30       30         Right Corner Area per Ped (sq.ft)       259.1       158.5       216.6       181.9         Right Corner Quality of Service       A       A       A       A         Ped. Circulation Area (sq.ft)       443.5       207.7       66.4       44.9         Crosswalk Circulation Code       A       A       A       B	Ped. R. Sidewalk Flow Rate (p/h)	0	0	0	0
Veh. RTOR Flow in Walk (v/h)322285th percentile speed (mph)30303030Right Corner Area per Ped (sq.ft)259.1158.5216.6181.9Right Corner Quality of ServiceAAAAPed. Circulation Area (sq.ft)443.5207.766.444.9Crosswalk Circulation CodeAAAB	Veh. Perm. L. Flow in Walk (v/h)	24	11	31	23
85th percentile speed (mph)       30       30       30       30         Right Corner Area per Ped (sq.ft)       259.1       158.5       216.6       181.9         Right Corner Quality of Service       A       A       A       A         Ped. Circulation Area (sq.ft)       443.5       207.7       66.4       44.9         Crosswalk Circulation Code       A       A       A       B	Veh. Perm. R. Flow in Walk (v/h)	29	15	14	23
Right Corner Area per Ped (sq.ft) 259.1 158.5 216.6 181.9 Right Corner Quality of Service A A A A A Ped. Circulation Area (sq.ft) 443.5 207.7 66.4 44.9 Crosswalk Circulation Code A A A B	Veh. RTOR Flow in Walk (v/h)	3	2	2	2
Right Corner Quality of Service A A A A A A A A A A A A A A A A A A A		30	30	30	30
Ped. Circulation Area (sq.ft) 443.5 207.7 66.4 44.9 Crosswalk Circulation Code A A B	Right Corner Area per Ped (sq.ft)	259.1	158.5	216.6	181.9
Ped. Circulation Area (sq.ft) 443.5 207.7 66.4 44.9 Crosswalk Circulation Code A A B	Right Corner Quality of Service	Α	Α	Α	Α
		443.5	207.7	66.4	44.9
		Α	Α	Α	В
Pedestrian Delay (s/p) 32.2 32.2 32.2 32.2	Pedestrian Delay (s/p)	32.2	32.2	32.2	32.2
• • • • • • • • • • • • • • • • • • • •	, , , ,	Poor	Poor	Poor	Poor
	·	2.41	2.38	1.83	1.82
Pedestrian Crosswalk LOS B B B	Pedestrian Crosswalk LOS	В	В	В	В

Approach		
Approach Direction	EB	
Median Present?	No	
Approach Delay(s)	15.6	
Level of Service	С	
Crosswalk		
Length (ft)	40	
Lanes Crossed	2	
Veh Vol Crossed	454	
Ped Vol Crossed	31	
Yield Rate(%)	50	
Ped Platooning	No	
r ca r latocrinig	140	
Critical Headway (s)	14.43	
Prob of Delayed X-ing	0.84	
Prob of Blocked Lane	0.60	
Delay for adq Gap	31.70	
Avg Ped Delay (s)	15.56	
Avy I du Delay (3)	13.30	
Approach		
Approach Direction	WB	
Median Present?	No	
Approach Delay(s)	15.6	
Level of Service	С	
Crosswalk		
Length (ft)	40	
Lanes Crossed	2	
Veh Vol Crossed	454	
Ped Vol Crossed	52	
Yield Rate(%)	50	
Ped Platooning	No	
	-1,0	
Critical Headway (s)	14.43	
Prob of Delayed X-ing	0.84	
Prob of Blocked Lane	0.60	
Delay for adq Gap	31.70	
Avg Ped Delay (s)	15.56	
	10.00	

Approach	
Approach Direction	EB
Median Present?	No
Approach Delay(s)	18.0
Level of Service	С
Crosswalk	
	50
Length (ft)	50
Lanes Crossed	2
Veh Vol Crossed	365
Ped Vol Crossed	25
Yield Rate(%)	50
Ped Platooning	No
Critical Headway (s)	17.29
Prob of Delayed X-ing	0.83
Prob of Blocked Lane	0.58
Delay for adq Gap	35.99
Avg Ped Delay (s)	18.04
Approach	
Approach Direction	WB
Median Present?	No
Approach Delay(s)	9.4
Level of Service	9.4 B
Level of Service	Б
Crosswalk	
Length (ft)	36
Lanes Crossed	2
Veh Vol Crossed	365
Ped Vol Crossed	21
Yield Rate(%)	50
Ped Platooning	No
Critical Headway (s)	13.29
Prob of Delayed X-ing	0.74
Prob of Blocked Lane	0.49
Delay for adq Gap	19.98
Avg Ped Delay (s)	9.41
	<b></b>

Approach	EB	WB	NB	SB
Crosswalk Length (ft)	60.0	60.0	40.0	40.0
Crosswalk Width (ft)	12.0	12.0	12.0	12.0
Total Number of Lanes Crossed	4	4	2	2
Number of Right-Turn Islands	0	0	0	0
Type of Control	Pretimed P	retimed P	retimed P	retimed
Corresponding Signal Phase	4	8	2	6
Effective Walk Time (s)	11.0	11.0	11.0	11.0
Right Corner Size A (ft)	9.0	9.0	9.0	9.0
Right Corner Size B (ft)	9.0	9.0	9.0	9.0
Right Corner Curb Radius (ft)	0.0	0.0	0.0	0.0
Right Corner Total Area (sq.ft)	81.00	81.00	81.00	81.00
Ped. Left-Right Flow Rate (p/h)	65	24	84	165
Ped. Right-Left Flow Rate (p/h)	65	25	84	166
Ped. R. Sidewalk Flow Rate (p/h)	0	0	0	0
Veh. Perm. L. Flow in Walk (v/h)	46	11	36	28
Veh. Perm. R. Flow in Walk (v/h)	33	14	12	23
Veh. RTOR Flow in Walk (v/h)	3	2	2	2
85th percentile speed (mph)	30	30	30	30
Right Corner Area per Ped (sq.ft)	224.5	171.7	315.9	138.0
Right Corner Quality of Service	А	Α	Α	Α
Ped. Circulation Area (sq.ft)	123.7	360.4	87.9	43.1
Crosswalk Circulation Code	А	Α	Α	В
Pedestrian Delay (s/p)	32.2	32.2	32.2	32.2
Pedestrian Compliance Code	Poor	Poor	Poor	Poor
Pedestrian Crosswalk Score	2.44	2.37	1.83	1.83
Pedestrian Crosswalk LOS	В	В	В	В

-		
Approach		
Approach Direction	EB	
Median Present?	No	
Approach Delay(s)	13.5	
Level of Service	С	
Crosswalk		
Length (ft)	40	
Lanes Crossed	2	
Veh Vol Crossed	409	
Ped Vol Crossed	29	
Yield Rate(%)	50	
Ped Platooning	No	
Critical Headway (s)	14.43	
Prob of Delayed X-ing	0.81	
Prob of Blocked Lane	0.56	
Delay for adq Gap	27.44	
Avg Ped Delay (s)	13.50	
Approach		
Approach Direction	WB	
Median Present?	No	
Approach Delay(s)	13.5	
Level of Service	C	
Crosswalk		
Length (ft)	40	
Lanes Crossed	2	
Veh Vol Crossed	409	
Ped Vol Crossed	28	
Yield Rate(%)	50	
Ped Platooning	No	
22.1.0009		
Critical Headway (s)	14.43	
Prob of Delayed X-ing	0.81	
Prob of Blocked Lane	0.56	
Delay for adq Gap	27.44	
Avg Ped Delay (s)	13.50	

Approach	
Approach Direction	EB
Median Present?	No
Approach Delay(s)	12.6
Level of Service	С
Crosswalk	
	40
Length (ft) Lanes Crossed	2
Veh Vol Crossed	392
Ped Vol Crossed	392
Yield Rate(%)	50 N-
Ped Platooning	No
	44.40
Critical Headway (s)	14.43
Prob of Delayed X-ing	0.79
Prob of Blocked Lane	0.54
Delay for adq Gap	25.98
Avg Ped Delay (s)	12.63
Approach	
Approach Direction	WB
Median Present?	No
Approach Delay(s)	12.6
Level of Service	C
	<u> </u>
Crosswalk	
Length (ft)	40
Lanes Crossed	2
Veh Vol Crossed	392
Ped Vol Crossed	14
Yield Rate(%)	50
Ped Platooning	No
Critical Headway (s)	14.43
Prob of Delayed X-ing	0.79
Prob of Blocked Lane	0.54
Delay for adq Gap	25.98
Avg Ped Delay (s)	12.63

# TRAFFIC SIGNAL WARRANTS – EXISTING (2022)





TRAFFIC SIGNAL WARRANTS		Existing Volumes (2022)							
		E Mendenhall Street & N Tracy Avenue	E Mendenhall Street & N Black Avenue	E Babcock Street & S Tracy Avenue	E Babcock Street & S Black Avenue				
1. Eight-Hour Vehicular V	olume	x	x	X	x				
2. Four-Hour Vehicular V	olume	x	x	X	X				
3. Peak Hour		x	x	X	X				
4. Pedestrian Volume		X	x	X	x				
5. School Crossing									
6. Coordinated Signal System		X	x	X	x				
7. Crash History		x	x	X	x				
8. Roadway Network		x	x	X	x				
9. Intersection Near a Grade Crossing									
Signals Warranted	Yes								
orginals warranted	No	X	х	x	X				

# Warrant 1: Eight-Hour Vehicular Volume

#### **General Information**

Agency/Company: Sanderson Stewart

Date: 9/27/2022 Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes): Mendenhall Street (2 lane)
Minor Street (Approach Lanes): Tracy Avenue (1 lane)
Analysis Year/Case: Existing (2022)

					Major Street	Higher Volume
Hour	Avg. Entering Volume			Total (Both	Minor	
Begin	NB	SB	EB	WB	Approaches)	Approach
0:00	3	7	0	29	29	7
1:00	3	3	0	21	21	3
2:00	1	0	0	7	7	1
3:00	0	1	0	6	6	1
4:00	1	3	0	8	8	3
5:00	14	8	0	27	27	14
6:00	8	9	0	66	66	9
7:00	40	34	0	182	182	40
8:00	43	40	0	293	293	43
9:00	82	41	0	296	296	82
10:00	103	67	0	318	318	103
11:00	86	77	0	409	409	86
12:00	130	77	0	488	488	130
13:00	89	75	0	444	444	89
14:00	97	71	0	385	385	97
15:00	72	65	0	417	417	72
16:00	76	62	0	422	422	76
17:00	73	56	0	514	514	73
18:00	84	75	0	335	335	84
19:00	64	45	0	259	259	64
20:00	42	29	0	240	240	42
21:00	36	30	0	194	194	36
22:00	23	16	0	122	122	23
23:00	11	8	0	65	65	11
TOTAL	1181	899	0	5547	5547	1189

Condition A - Minimum Vehicular Volume (100% Columns):		Hrs
Major Street Total > 600 and Higher Minor Street Total > 150 for 8 hours?	No	0
Condition B - Interruption of Continuous Traffic (100% Columns):		
Major Street Total > 900 and Higher Minor Street Total > 75 for 8 hours?	No	0
Combination of Conditions A & B (80% Columns):		
Major Street Total > 480 and Higher Minor Street Total > 120 for 8 hours?	No	1
Major Street Total > 720 and Higher Minor Street Total > 60 for 8 hours?	No	0
Warrant 1 Satisfied?	No	

#### Warrant 2: Four-Hour Vehicular Volume

#### **General Information**

Agency/Company: Sanderson Stewart
Date: 9/27/2022

Project Number: 18098.33

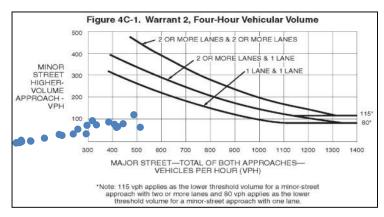
Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes): Mendenhall Street (2 lane)
Minor Street (Approach Lanes): Tracy Avenue (1 lane)
Analysis Year/Case: Existing (2022)

Hour		Avg. Enter	ing Volume	Major Street Total (Both	Higher Volume Minor	
Begin	NB	SB	EB	WB	Approaches)	Approach
0:00	3	7	0	29	29	7
1:00	3	3	0	21	21	3
2:00	1	0	0	7	7	1
3:00	0	1	0	6	6	1
4:00	1	3	0	8	8	3
5:00	14	8	0	27	27	14
6:00	8	9	0	66	66	9
7:00	40	34	0	182	182	40
8:00	43	40	0	293	293	43
9:00	82	41	0	296	296	82
10:00	103	67	0	318	318	103
11:00	86	77	0	409	409	86
12:00	130	77	0	488	488	130
13:00	89	75	0	444	444	89
14:00	97	71	0	385	385	97
15:00	72	65	0	417	417	72
16:00	76	62	0	422	422	76
17:00	73	56	0	514	514	73
18:00	84	75	0	335	335	84
19:00	64	45	0	259	259	64
20:00	42	29	0	240	240	42
21:00	36	30	0	194	194	36
22:00	23	16	0	122	122	23
23:00	11	8	0	65	65	11
TOTAL	1181	899	0	5547	5547	1189



Meets warrant criteria on graph for minimum of 4 hours (100% thresholds)? Warrant 2 Satisfied?

No (0 hrs)

#### Warrant 3: Peak Hour

#### General Information

Agency/Company: Sanderson Stewart Date: 9/27/2022

Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes): Mendenhall Street (2 lane) Minor Street (Approach Lanes): Tracy Avenue (1 lane) Analysis Year/Case: Existing (2022)

High Minor Total Stopped

$\mathbf{A}\mathbf{M}$	Peak Hour	

ligh Minor Total Stopped Time Delay (hrs)	0.21
Total Volume of Major Approaches (vehs)	307
High Minor Approach Volume (vehs)	60
Total Entering Volume (vehs)	367

PM Peak Hour

4:45 -	5:45	PM	
--------	------	----	--

8:30 - 9:30 AM

High Minor Total Stopped Time Delay (hrs) Total Volume of Major Approaches (vehs) High Minor Approach Volume (vehs) Total Entering Volume (vehs)

0.61
487
121
608

Category A: Peak Period: AM

Total stopped time delay for minor approach > 4 veh-hrs? High minor approach volume > 100 for peak hour?

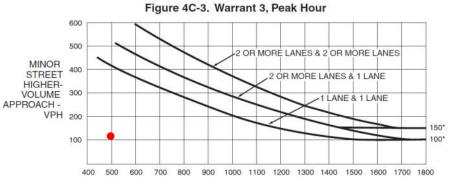
Total entering volume > 800 for peak hour?

No (608) No

No (0.61) Yes (121)

Category B:

Category A warrant satisfied?



MAJOR STREET-TOTAL OF BOTH APPROACHES-VEHICLES PER HOUR (VPH)

\*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Meets warrant criteria on graph for minimum of one hour (100% thresholds)?

No

Warrant 3 Satisfied?

#### Warrant 4: Pedestrian Volume

#### General Information

Agency/Company: Sanderson Stewart

Date: 9/27/2022 Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

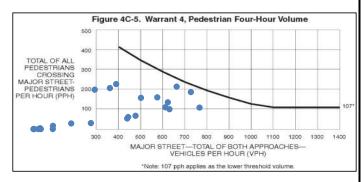
Major Street Speed Limit: 25 mph

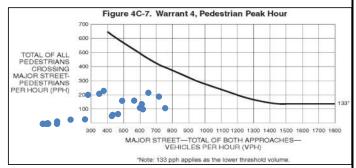
Major Street (Approach Lanes): Mendenhall Street (2 lane)
Minor Street (Approach Lanes): Tracy Avenue (1 lane)
Analysis Year/Case: Design Year (2042)

This warrant is intended for application where the traffic volume on a major street is so heavy that pedestrians

experience excessive delay in crossing the major street.

Hour Begin	Major Street Total Traffic	Pedestrian Volume Crossing Major Street
0:00	43	0
1:00	31	0
2:00	10	0
3:00	9	0
4:00	12	0
5:00	40	4
6:00	98	16
7:00	270	30
8:00	435	53
9:00	440	61
10:00	473	67
11:00	608	110
12:00	725	187
13:00	660	214
14:00	572	160
15:00	620	135
16:00	627	100
17:00	764	108
18:00	498	158
19:00	385	227
20:00	357	207
21:00	288	199
22:00	181	28
23:00	97	0
TOTAL	8,243	2064





For each of any 4 hours of an average day, do the plotted points representing representing the vehicles per hour on the major street and the corresponding pedestrians per hour crossing the major street fall above the curve in Figure 4C-5?

No

For 1 hour of an average day, does the plotted point representing vehicles per hour on the major street and the corresponding pedestrians per hour crossing the major street fall above the curve in Figure 4C-7?

No

Warrant 4 Satisfied?

Agency/Company: Sanderson Stewart

 Date:
 9/27/2022

 Project Number:
 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes): Mendenhall Street (2 lane)
Minor Street (Approach Lanes): Tracy Avenue (1 lane)
Analysis Year/Case: Existing (2022)

#### Warrant 5: School Crossing

This warrant is intended for application where the fact that school children (elementary through high school students) cross the major street is the principle reason to consider installing a traffic signal. This warrant shall not be applied at locations where the distance to the nearest traffic control signal along the major street is less than 300 feet, unless it can be shown that the proposed traffic signal would not restrict the progressive movement of traffic.

Is the number of adequate gaps in the major crossing traffic steam during the primary crossing period less than the number of minutes in that crossing period?

N/A

Do 20 or more students cross at this location during the highest crossing hour? N/A

Warrant 5 Satisfied? N/A

#### Warrant 6: Coordinated Signal System

This warrant is intended for application where installation of a traffic signal would help to provide proper platooning of vehicles and therefore provide progressive movement in a coordinated signal system.

Are any adjacent traffic signals located so far away that they do not provide a necessary degree of platooning and/or progressive operation?

No

Warrant 6 Satisfied? No

#### Warrant 7: Crash Experience

This warrant is intended for application where the severity and frequency of crashes are the principal reasons to consider installing a traffic control signal

Have adequate trials of alternatives failed to reduce the crash frequency? N/A

Have 5 or more crashes, of types susceptible to correction by a signal, occurred within a 12-month period?

Is Condition A criterion met for 80% columns of Warrant 1 met?

Is Condition B criterion met for 80% columns of Warrant 1 met?

Are observed pedestrian volumes equal to or greater than 80% of what is required for Warrant 4?

No

Warrant 7 Satisfied?

Agency/Company: Sanderson Stewart

Date: 9/27/2022

Project Number: 18008 32

Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes): Mendenhall Street (2 lane)
Minor Street (Approach Lanes): Tracy Avenue (1 lane)
Analysis Year/Case: Existing (2022)

#### Warrant 8: Roadway Network

This warrant is intended for application where installation of a traffic signal could be justified in order to encourage concentration and organization of traffic flow on a roadway network

Do two or more of the intersecting routes at this location have at least one of the following characteristics:

- A. It is part of the street or highway system that serves as the principal roadway network for through traffic flow; or
- B. It includes rural or suburban highways outside, entering, or traversing a City; or
- C. It appears as a major route on an official plan.

No

Does this intersection have an existing or immediately projected total entering volume of a least 1000 vehicles during a weekday typical peak hour and have a 5-year projected traffic volume that meets one or more of Warrants 1, 2, and 3 during an average weekday?

No

Does this intersection have an existing or immediately projected total entering volume of at least 1000 vph for each of any 5 hours of a Saturday or Sunday?

N/A

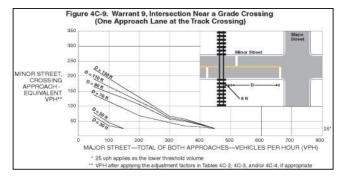
Warrant 8 Satisfied?

#### Warrant 9: Intersection Near a Grade Crossing

This warrant is intended for application where none of the conditions described in the other eight traffic signal warrants are met, but the proximity to the intersection of a grade crossing on an intersection approach controlled by a STOP or YIELD sign is the principal reason to consider installing a traffic signal.

Does a grade crossing exist on an approach controlled by a STOP or YIELD sign whereby the center of the track nearest to the intersection is within 140 feet of the stop or yield line?

No



During the highest traffic volume hour during which the rail traffic uses the crossing, does the plotted point representing vehicles per hour on the major street and the corresponding vehicles per hour on the minor-street approach that crosses the track fall above the applicable curve in Figure 4C-9 or 4C-10 (whichever is applicable) for the existing combination of approach lanes over the track and the distance D, which is the clear storage distance?

N/A

Warrant 9 Satisfied?

N/A

# Warrant 1: Eight-Hour Vehicular Volume

#### **General Information**

Agency/Company: Sanderson Stewart

Date: 9/27/2022 Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes):
Minor Street (Approach Lanes):
Analysis Year/Case:

Mendenhall Street (2 lane)
Black Avenue (1 lane)
Existing (2022)

Hour		A E			Major Street Total (Both	Higher Volume Minor
Begin	NB	Avg. Enteri SB	EB	WB	Approaches)	Approach
0:00	5	6	0	20	20	6
1:00	3	2	0	15	15	3
2:00	1	0	0	6	6	1
3:00	0	1	0	5	5	1
4:00	1	6	0	5	5	6
5:00	6	7	0	24	24	7
6:00	15	19	0	63	63	19
7:00	36	37	0	172	172	37
8:00	68	94	0	268	268	94
9:00	64	57	0	287	287	64
10:00	86	71	0	307	307	86
11:00	104	69	0	421	421	104
12:00	114	91	0	450	450	114
13:00	102	76	0	390	390	102
14:00	90	50	0	359	359	90
15:00	96	63	0	383	383	96
16:00	66	69	0	372	372	69
17:00	78	74	0	454	454	78
18:00	62	97	0	309	309	97
19:00	42	64	0	213	213	64
20:00	34	37	0	182	182	37
21:00	28	27	0	143	143	28
22:00	20	23	0	99	99	23
23:00	10	5	0	60	60	10
TOTAL	1131	1045	0	5007	5007	1236

Condition A - Minimum Vehicular Volume (100% Columns):		Hrs
Major Street Total > 600 and Higher Minor Street Total > 150 for 8 hours?	No	0
Condition B - Interruption of Continuous Traffic (100% Columns):		
Major Street Total > 900 and Higher Minor Street Total > 75 for 8 hours?	No	0
Combination of Conditions A & B (80% Columns):		
Major Street Total > 480 and Higher Minor Street Total > 120 for 8 hours?	No	0
Major Street Total > 720 and Higher Minor Street Total > 60 for 8 hours?	No	0
Warrant 1 Satisfied?	No	

#### Warrant 2: Four-Hour Vehicular Volume

#### **General Information**

Agency/Company: Sanderson Stewart
Date: 9/27/2022

Project Number: 18098.33

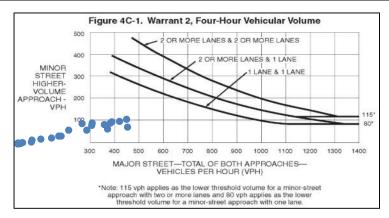
Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes): Mendenhall Street (2 lane)
Minor Street (Approach Lanes): Black Avenue (1 lane)
Analysis Year/Case: Existing (2022)

Hour		Avg. Enteri	ng Volume		Major Street Total (Both	Higher Volume Minor
Begin	NB	SB	EB	WB	Approaches)	Approach
0:00	5	6	0	20	20	6
1:00	3	2	0	15	15	3
2:00	1	0	0	6	6	1
3:00	0	1	0	5	5	1
4:00	1	6	0	5	5	6
5:00	6	7	0	24	24	7
6:00	15	19	0	63	63	19
7:00	36	37	0	172	172	37
8:00	68	94	0	268	268	94
9:00	64	57	0	287	287	64
10:00	86	71	0	307	307	86
11:00	104	69	0	421	421	104
12:00	114	91	0	450	450	114
13:00	102	76	0	390	390	102
14:00	90	50	0	359	359	90
15:00	96	63	0	383	383	96
16:00	66	69	0	372	372	69
17:00	78	74	0	454	454	78
18:00	62	97	0	309	309	97
19:00	42	64	0	213	213	64
20:00	34	37	0	182	182	37
21:00	28	27	0	143	143	28
22:00	20	23	0	99	99	23
23:00	10	5	0	60	60	10
TOTAL	1131	1045	0	5007	5007	1236



Meets warrant criteria on graph for minimum of 4 hours (100% thresholds)? Warrant 2 Satisfied?

No (0 hrs)

#### Warrant 3: Peak Hour

#### General Information

Agency/Company: Sanderson Stewart Date: 9/27/2022

Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes): Mendenhall Street (2 lane) Minor Street (Approach Lanes): Black Avenue (1 lane) Analysis Year/Case: Existing (2022)

AM Peak Hour I	M	Peak	Hour	
----------------	---	------	------	--

·	
High Minor Total Stopped Time Delay (hrs)	0.33
Total Volume of Major Approaches (vehs)	288
High Minor Approach Volume (vehs)	90
Total Entering Volume (vehs)	378

PM Peak Hour

8:30 - 9:30 AM

High Minor Total Stopped Time Delay (hrs) Total Volume of Major Approaches (vehs) High Minor Approach Volume (vehs) Total Entering Volume (vehs)

0.37
457
78
535

Category A: Peak Period: AM

Total stopped time delay for minor approach > 4 veh-hrs? High minor approach volume > 100 for peak hour? Total entering volume > 800 for peak hour?

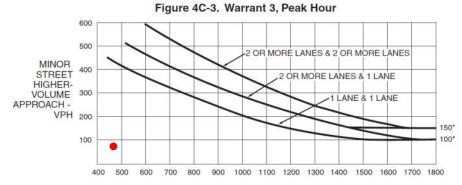
No (0.37) No (78)

No (535)

Category A warrant satisfied?

No





MAJOR STREET-TOTAL OF BOTH APPROACHES-VEHICLES PER HOUR (VPH)

\*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Meets warrant criteria on graph for minimum of one hour (100% thresholds)?

No

Warrant 3 Satisfied?

#### Warrant 4: Pedestrian Volume

#### **General Information**

Agency/Company: Sanderson Stewart

Date: 9/27/2022 Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

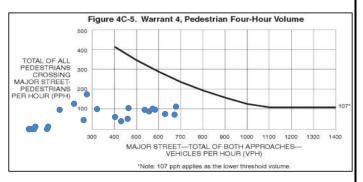
Major Street Speed Limit: 25 mph

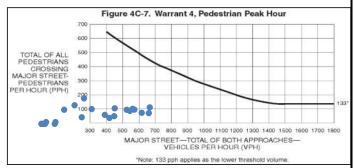
Major Street (Approach Lanes): Mendenhall Street (2 lane)
Minor Street (Approach Lanes): Black Avenue (1 lane)
Analysis Year/Case: Design Year (2042)

This warrant is intended for application where the traffic volume on a major street is so heavy that pedestrians

experience excessive delay in crossing the major street.

Hour Begin	Major Street Total Traffic	Pedestrian Volume Crossing Major Street
0:00	30	0
1:00	22	0
2:00	9	0
3:00	7	0
4:00	7	0
5:00	36	12
6:00	94	12
7:00	256	45
8:00	398	61
9:00	426	40
10:00	456	52
11:00	626	76
12:00	669	73
13:00	580	98
14:00	533	97
15:00	569	103
16:00	553	89
17:00	675	113
18:00	459	106
19:00	317	101
20:00	270	175
21:00	212	128
22:00	147	97
23:00	89	0
TOTAL	7,440	1478





For each of any 4 hours of an average day, do the plotted points representing representing the vehicles per hour on the major street and the corresponding pedestrians per hour crossing the major street fall above the curve in Figure 4C-5?

No

For 1 hour of an average day, does the plotted point representing vehicles per hour on the major street and the corresponding pedestrians per hour crossing the major street fall above the curve in Figure 4C-7?

No

Warrant 4 Satisfied?

Agency/Company: Sanderson Stewart

Date: 9/27/2022 Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes): Mendenhall Street (2 lane)
Minor Street (Approach Lanes): Black Avenue (1 lane)
Analysis Year/Case: Existing (2022)

#### Warrant 5: School Crossing

This warrant is intended for application where the fact that school children (elementary through high school students) cross the major street is the principle reason to consider installing a traffic signal. This warrant shall not be applied at locations where the distance to the nearest traffic control signal along the major street is less than 300 feet, unless it can be shown that the proposed traffic signal would not restrict the progressive movement of traffic.

Is the number of adequate gaps in the major crossing traffic steam during the primary crossing period less than the number of minutes in that crossing period?

N/A

Do 20 or more students cross at this location during the highest crossing hour? N/A

Warrant 5 Satisfied? N/A

#### Warrant 6: Coordinated Signal System

This warrant is intended for application where installation of a traffic signal would help to provide proper platooning of vehicles and therefore provide progressive movement in a coordinated signal system.

Are any adjacent traffic signals located so far away that they do not provide a necessary degree of platooning and/or progressive operation?

No

Warrant 6 Satisfied? No

#### Warrant 7: Crash Experience

This warrant is intended for application where the severity and frequency of crashes are the principal reasons to consider installing a traffic control signal

Have adequate trials of alternatives failed to reduce the crash frequency?

N/A

Have 5 or more crashes, of types susceptible to correction by a signal, occurred within a 12-month period?

Is Condition A criterion met for 80% columns of Warrant 1 met?

Is Condition B criterion met for 80% columns of Warrant 1 met?

Are observed pedestrian volumes equal to or greater than 80% of what is required for Warrant 4?

No

Warrant 7 Satisfied?

Agency/Company: Sanderson Stewart
Date: 9/27/2022
Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes): Mendenhall Street (2 lane)
Minor Street (Approach Lanes): Black Avenue (1 lane)
Analysis Year/Case: Existing (2022)

#### Warrant 8: Roadway Network

This warrant is intended for application where installation of a traffic signal could be justified in order to encourage concentration and organization of traffic flow on a roadway network

Do two or more of the intersecting routes at this location have at least one of the following characteristics:

- A. It is part of the street or highway system that serves as the principal roadway network for through traffic flow; or
- B. It includes rural or suburban highways outside, entering, or traversing a City; or
- C. It appears as a major route on an official plan.

No

Does this intersection have an existing or immediately projected total entering volume of a least 1000 vehicles during a weekday typical peak hour and have a 5-year projected traffic volume that meets one or more of Warrants 1, 2, and 3 during an average weekday?

No

Does this intersection have an existing or immediately projected total entering volume of at least 1000 vph for each of any 5 hours of a Saturday or Sunday?

N/A

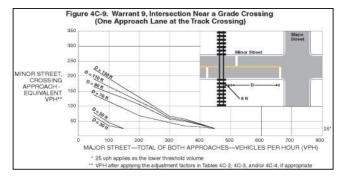
Warrant 8 Satisfied?

#### Warrant 9: Intersection Near a Grade Crossing

This warrant is intended for application where none of the conditions described in the other eight traffic signal warrants are met, but the proximity to the intersection of a grade crossing on an intersection approach controlled by a STOP or YIELD sign is the principal reason to consider installing a traffic signal.

Does a grade crossing exist on an approach controlled by a STOP or YIELD sign whereby the center of the track nearest to the intersection is within 140 feet of the stop or yield line?

No



During the highest traffic volume hour during which the rail traffic uses the crossing, does the plotted point representing vehicles per hour on the major street and the corresponding vehicles per hour on the minor-street approach that crosses the track fall above the applicable curve in Figure 4C-9 or 4C-10 (whichever is applicable) for the existing combination of approach lanes over the track and the distance D, which is the clear storage distance?

N/A

Warrant 9 Satisfied?

N/A

# Warrant 1: Eight-Hour Vehicular Volume

#### General Information

Agency/Company: Sanderson Stewart

Date: 9/27/2022 Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes):

Minor Street (Approach Lanes):

Analysis Year/Case:

Babcock Street (2 lane)

Black Avenue (1 lane)

Existing (2022)

Hour		Avg. Enteri	na Volume		Major Street Total (Both	Higher Volume Minor
Begin	NB	SB	EB	WB	Approaches)	Approach
0:00	2	0	8	0	8	2
1:00	0	3	8	0	8	3
2:00	2	0	3	0	3	2
3:00	0	1	3	0	3	1
4:00	1	1	8	0	8	1
5:00	7	5	46	0	46	7
6:00	13	24	90	0	90	24
7:00	23	35	269	0	269	35
8:00	31	76	383	0	383	76
9:00	58	91	352	0	352	91
10:00	61	117	358	0	358	117
11:00	84	124	430	0	430	124
12:00	93	127	502	0	502	127
13:00	78	107	433	0	433	107
14:00	67	97	378	0	378	97
15:00	76	84	393	0	393	84
16:00	68	56	371	0	371	68
17:00	52	71	402	0	402	71
18:00	33	60	289	0	289	60
19:00	23	33	194	0	194	33
20:00	16	28	128	0	128	28
21:00	11	19	75	0	75	19
22:00	4	7	44	0	44	7
23:00	6	10	19	0	19	10
TOTAL	809	1176	5186	0	5186	1194

Condition A - Minimum Vehicular Volume (100% Columns):		Hrs
Major Street Total > 600 and Higher Minor Street Total > 150 for 8 hours?	No	0
Condition B - Interruption of Continuous Traffic (100% Columns):		
Major Street Total > 900 and Higher Minor Street Total > 75 for 8 hours?	No	0
Combination of Conditions A & B (80% Columns):		
Major Street Total > 480 and Higher Minor Street Total > 120 for 8 hours?	No	1
Major Street Total > 720 and Higher Minor Street Total > 60 for 8 hours?	No	0
Warrant 1 Satisfied?	No	

#### Warrant 2: Four-Hour Vehicular Volume

#### **General Information**

Agency/Company: Sanderson Stewart
Date: 9/27/2022

Project Number: 18098.33

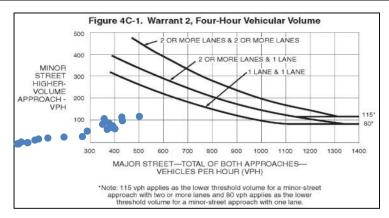
Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes): Babcock Street (2 lane)
Minor Street (Approach Lanes): Black Avenue (1 lane)
Analysis Year/Case: Existing (2022)

Hour		Avg. Enteri	Major Street Total (Both	Higher Volume Minor		
Begin	NB	SB	EB	WB	Approaches)	Approach
0:00	2	0	8	0	8	2
1:00	0	3	8	0	8	3
2:00	2	0	3	0	3	2
3:00	0	1	3	0	3	1
4:00	1	1	8	0	8	1
5:00	7	5	46	0	46	7
6:00	13	24	90	0	90	24
7:00	23	35	269	0	269	35
8:00	31	76	383	0	383	76
9:00	58	91	352	0	352	91
10:00	61	117	358	0	358	117
11:00	84	124	430	0	430	124
12:00	93	127	502	0	502	127
13:00	78	107	433	0	433	107
14:00	67	97	378	0	378	97
15:00	76	84	393	0	393	84
16:00	68	56	371	0	371	68
17:00	52	71	402	0	402	71
18:00	33	60	289	0	289	60
19:00	23	33	194	0	194	33
20:00	16	28	128	0	128	28
21:00	11	19	75	0	75	19
22:00	4	7	44	0	44	7
23:00	6	10	19	0	19	10
TOTAL	809	1176	5186	0	5186	1194



Meets warrant criteria on graph for minimum of 4 hours (100% thresholds)? Warrant 2 Satisfied?

No (0 hrs)

#### Warrant 3: Peak Hour

#### General Information

Agency/Company: Sanderson Stewart
Date: 9/27/2022

Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes):

Minor Street (Approach Lanes):

Analysis Year/Case:

Babcock Street (2 lane)

Black Avenue (1 lane)

Existing (2022)

#### AM Peak Hour

Hour	8:30 - 9:30 AM					
(hrs)	0.31					
(vehs)	385					
(vehs)	83					
	460					

High Minor Total Stopped Time Delay (hrs)
Total Volume of Major Approaches (vehs)
High Minor Approach Volume (vehs)
Total Entering Volume (vehs)

PM Peak Hour

4:45	-	5:45	PM	

High Minor Total Stopped Time Delay (hrs)
Total Volume of Major Approaches (vehs)
High Minor Approach Volume (vehs)
Total Entering Volume (vehs)

0.28
404
72
476

Category A: Peak Period: AM

Total stopped time delay for minor approach > 4 veh-hrs? High minor approach volume > 100 for peak hour?

No (0.31) No (72)

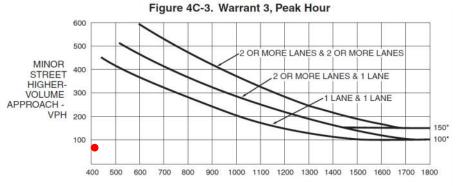
No (476)

Total entering volume > 800 for peak hour?

No

Category A warrant satisfied?





MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

\*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Meets warrant criteria on graph for minimum of one hour (100% thresholds)?

No

Warrant 3 Satisfied?

#### Warrant 4: Pedestrian Volume

#### General Information

Agency/Company: Sanderson Stewart

Date: 9/27/2022 Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

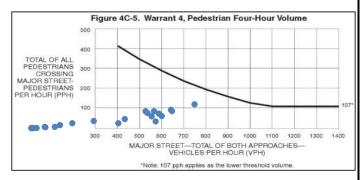
Major Street Speed Limit: 25 mph

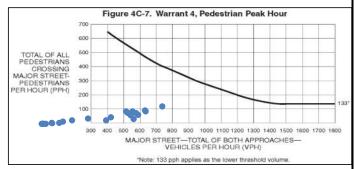
Major Street (Approach Lanes): Babcock Street (2 lane)
Minor Street (Approach Lanes): Black Avenue (1 lane)
Analysis Year/Case: Design Year (2042)

This warrant is intended for application where the traffic volume on a major street is so heavy that pedestrians

experience excessive delay in crossing the major street.

Hour Begin	Major Street Total Traffic	Pedestrian Volume Crossing Major Street
0:00	12	0
1:00	12	0
2:00	4	0
3:00	4	0
4:00	12	0
5:00	68	4
6:00	134	15
7:00	400	24
8:00	569	33
9:00	523	85
10:00	532	76
11:00	639	92
12:00	746	120
13:00	643	86
14:00	562	85
15:00	584	73
16:00	551	59
17:00	597	61
18:00	429	45
19:00	288	36
20:00	190	24
21:00	111	6
22:00	65	6
23:00	28	0
TOTAL	7,703	930





For each of any 4 hours of an average day, do the plotted points representing representing the vehicles per hour on the major street and the corresponding pedestrians per hour crossing the major street fall above the curve in Figure 4C-5?

No

For 1 hour of an average day, does the plotted point representing vehicles per hour on the major street and the corresponding pedestrians per hour crossing the major street fall above the curve in Figure 4C-7?

No

Warrant 4 Satisfied?

Agency/Company: Sanderson Stewart

Date: 9/27/2022 Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes):

Minor Street (Approach Lanes):

Analysis Year/Case:

Babcock Street (2 lane)

Black Avenue (1 lane)

Existing (2022)

#### Warrant 5: School Crossing

This warrant is intended for application where the fact that school children (elementary through high school students) cross the major street is the principle reason to consider installing a traffic signal. This warrant shall not be applied at locations where the distance to the nearest traffic control signal along the major street is less than 300 feet, unless it can be shown that the proposed traffic signal would not restrict the progressive movement of traffic.

Is the number of adequate gaps in the major crossing traffic steam during the primary crossing period less than the number of minutes in that crossing period?

N/A

Do 20 or more students cross at this location during the highest crossing hour? N/A

Warrant 5 Satisfied? N/A

#### Warrant 6: Coordinated Signal System

This warrant is intended for application where installation of a traffic signal would help to provide proper platooning of vehicles and therefore provide progressive movement in a coordinated signal system.

Are any adjacent traffic signals located so far away that they do not provide a necessary degree of platooning and/or progressive operation?  ${
m No}$ 

Warrant 6 Satisfied? No

#### Warrant 7: Crash Experience

This warrant is intended for application where the severity and frequency of crashes are the principal reasons to consider installing a traffic control signal

Have adequate trials of alternatives failed to reduce the crash frequency? N/A

Have 5 or more crashes, of types susceptible to correction by a signal, occurred within a 12-month period?

Is Condition A criterion met for 80% columns of Warrant 1 met?

Is Condition B criterion met for 80% columns of Warrant 1 met?

Are observed pedestrian volumes equal to or greater than 80% of what is required for Warrant 4?

No

Warrant 7 Satisfied?

Agency/Company: Sanderson Stewart
Date: 9/27/2022
Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes):

Minor Street (Approach Lanes):

Analysis Year/Case:

Babcock Street (2 lane)

Black Avenue (1 lane)

Existing (2022)

#### Warrant 8: Roadway Network

This warrant is intended for application where installation of a traffic signal could be justified in order to encourage concentration and organization of traffic flow on a roadway network

Do two or more of the intersecting routes at this location have at least one of the following characteristics:

- A. It is part of the street or highway system that serves as the principal roadway network for through traffic flow; or
- B. It includes rural or suburban highways outside, entering, or traversing a City; or
- C. It appears as a major route on an official plan.

No

Does this intersection have an existing or immediately projected total entering volume of a least 1000 vehicles during a weekday typical peak hour and have a 5-year projected traffic volume that meets one or more of Warrants 1, 2, and 3 during an average weekday?

No

Does this intersection have an existing or immediately projected total entering volume of at least 1000 vph for each of any 5 hours of a Saturday or Sunday?

N/A

Warrant 8 Satisfied?

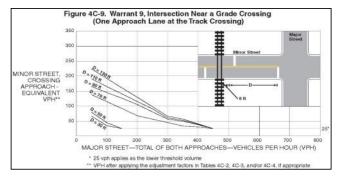
No

#### Warrant 9: Intersection Near a Grade Crossing

This warrant is intended for application where none of the conditions described in the other eight traffic signal warrants are met, but the proximity to the intersection of a grade crossing on an intersection approach controlled by a STOP or YIELD sign is the principal reason to consider installing a traffic signal.

Does a grade crossing exist on an approach controlled by a STOP or YIELD sign whereby the center of the track nearest to the intersection is within 140 feet of the stop or yield line?

No



During the highest traffic volume hour during which the rail traffic uses the crossing, does the plotted point representing vehicles per hour on the major street and the corresponding vehicles per hour on the minor-street approach that crosses the track fall above the applicable curve in Figure 4C-9 or 4C-10 (whichever is applicable) for the existing combination of approach lanes over the track and the distance D, which is the clear storage distance?

N/A

Warrant 9 Satisfied?

N/A

# Warrant 1: Eight-Hour Vehicular Volume

#### General Information

Agency/Company: Sanderson Stewart

Date: 9/27/2022 Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes):

Minor Street (Approach Lanes):

Analysis Year/Case:

Babcock Street (2 lane)

Black Avenue (1 lane)

Existing (2022)

Hour		Arro Entoni	m o Volumo		Major Street Total (Both	Higher Volume Minor
Begin	NB	Avg. Enteri SB	EB	WB	Approaches)	Approach
0:00	0	2	9	0	9	2
1:00	0	2	8	0	8	2
2:00	0	1	3	0	3	1
3:00	0	0	3	0	3	0
4:00	0	0	5	0	5	0
5:00	3	2	54	0	54	3
6:00	13	11	105	0	105	13
7:00	22	20	278	0	278	22
8:00	34	36	405	0	405	36
9:00	54	36	369	0	369	54
10:00	58	72	409	0	409	72
11:00	56	70	507	0	507	70
12:00	74	63	553	0	553	74
13:00	70	74	475	0	475	74
14:00	59	71	410	0	410	71
15:00	53	54	449	0	449	54
16:00	60	62	408	0	408	62
17:00	70	86	447	0	447	86
18:00	44	43	298	0	298	44
19:00	23	41	210	0	210	41
20:00	10	20	128	0	128	20
21:00	10	16	80	0	80	16
22:00	7	8	45	0	45	8
23:00	3	6	19	0	19	6
TOTAL	723	796	5677	0	5677	831

Condition A - Minimum Vehicular Volume (100% Columns):		Hrs
Major Street Total > 600 and Higher Minor Street Total > 150 for 8 hours?	No	0
Condition B - Interruption of Continuous Traffic (100% Columns):		
Major Street Total > 900 and Higher Minor Street Total > 75 for 8 hours?	No	0
Combination of Conditions A & B (80% Columns):		
Major Street Total > 480 and Higher Minor Street Total > 120 for 8 hours?	No	0
Major Street Total > 720 and Higher Minor Street Total > 60 for 8 hours?	No	0
Warrant 1 Satisfied?	No	

#### Warrant 2: Four-Hour Vehicular Volume

#### **General Information**

Agency/Company: Sanderson Stewart
Date: 9/27/2022

Project Number: 18098.33

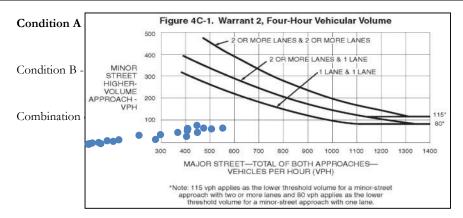
Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes): Babcock Street (2 lane)
Minor Street (Approach Lanes): Black Avenue (1 lane)
Analysis Year/Case: Existing (2022)

Hour		Avg. Enteri	Major Street Total (Both	Higher Volume Minor		
Begin	NB	SB	EB	WB	Approaches)	Approach
0:00	0	2	9	0	9	2
1:00	0	2	8	0	8	2
2:00	0	1	3	0	3	1
3:00	0	0	3	0	3	0
4:00	0	0	5	0	5	0
5:00	3	2	54	0	54	3
6:00	13	11	105	0	105	13
7:00	22	20	278	0	278	22
8:00	34	36	405	0	405	36
9:00	54	36	369	0	369	54
10:00	58	72	409	0	409	72
11:00	56	70	507	0	507	70
12:00	74	63	553	0	553	74
13:00	70	74	475	0	475	74
14:00	59	71	410	0	410	71
15:00	53	54	449	0	449	54
16:00	60	62	408	0	408	62
17:00	70	86	447	0	447	86
18:00	44	43	298	0	298	44
19:00	23	41	210	0	210	41
20:00	10	20	128	0	128	20
21:00	10	16	80	0	80	16
22:00	7	8	45	0	45	8
23:00	3	6	19	0	19	6
TOTAL	723	796	5677	0	5677	831



Meets warrant criteria on graph for minimum of 4 hours (100% thresholds)? Warrant 2 Satisfied?

No (0 hrs)

#### Warrant 3: Peak Hour

#### General Information

Agency/Company: Sanderson Stewart Date: 9/27/2022

Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes): Babcock Street (2 lane) Minor Street (Approach Lanes): Black Avenue (1 lane) Analysis Year/Case: Existing (2022)

#### AM Peak Hour

our	8:30 - 9:30 AM							
nrs)	0.20							
hs)	402							
hs)	52							

High Minor Total Stopped Time Delay (h Total Volume of Major Approaches (ve High Minor Approach Volume (ve Total Entering Volume (vehs)

PM Peak Hour

0.39	
445	
88	
533	

4:45 - 5:45 PM

High Minor Total Stopped Time Delay (hrs) Total Volume of Major Approaches (vehs) High Minor Approach Volume (vehs) Total Entering Volume (vehs)

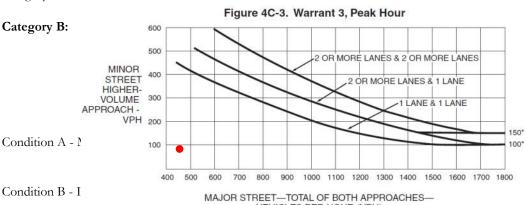
Category A: Peak Period: AM

Total stopped time delay for minor approach > 4 veh-hrs? No (0.39) No (88) High minor approach volume > 100 for peak hour? Total entering volume > 800 for peak hour? No (533)

Category A warrant satisfied?



No



VEHICLES PER HOUR (VPH)

\*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane. Combination of

Meets warrant criteria on graph for minimum of one hour (100% thresholds)?

Warrant 3 Satisfied? No

#### Warrant 4: Pedestrian Volume

#### **General Information**

Agency/Company: Sanderson Stewart

Date: 9/27/2022 Project Number: 18098 33

Project Description: Babcock and Mendenhall Safety Improvements

Iurisdiction: City of Bozeman/MDT

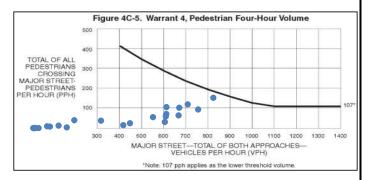
Major Street Speed Limit: 25 mph

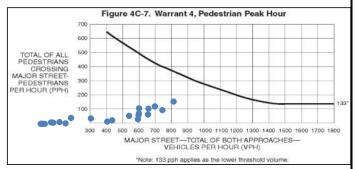
Major Street (Approach Lanes): Babcock Street (2 lane) Minor Street (Approach Lanes): Black Avenue (1 lane) Analysis Year/Case: Design Year (2042)

This warrant is intended for application where the traffic volume on a major street is so heavy that pedestrians

experience excessive delay in crossing the major street.

	Hour Begin	Major Street Total Traffic	Pedestrian Volume Crossing Major Street
	0:00	13	0
	1:00	12	0
	2:00	4	0
	3:00	4	0
	4:00	7	0
	5:00	80	7
	6:00	156	3
	7:00	413	15
	8:00	602	31
	9:00	548	55
	10:00	608	71
	11:00	753	94
	12:00	822	153
	13:00	706	120
	14:00	609	106
	15:00	667	103
	16:00	606	59
	17:00	664	64
	18:00	443	24
	19:00	312	37
	20:00	190	40
	21:00	119	12
	22:00	67	9
	23:00	28	0
um '	Vehicular	8,433	1003





For each of any 4 hours of an average day, do the plotted points representing representing the vehicles per hour on the major street and the corresponding pedestrians per hour crossing the major street fall above the curve in Figure 4C-5? No

For 1 hour of an average day, does the plotted point representing vehicles per hour on the major street and the corresponding pedestrians per hour crossing No

the major street fall above the curve in Figure 4C-7? Condition B - Interruption of Continuous Traffic (100% Columns):

Warrant 4 Satisfied?

Agency/Company: Sanderson Stewart

Date: 9/27/2022 Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes):

Minor Street (Approach Lanes):

Analysis Year/Case:

Babcock Street (2 lane)

Black Avenue (1 lane)

Existing (2022)

#### Warrant 5: School Crossing

This warrant is intended for application where the fact that school children (elementary through high school students) cross the major street is the principle reason to consider installing a traffic signal. This warrant shall not be applied at locations where the distance to the nearest traffic control signal along the major street is less than 300 feet, unless it can be shown that the proposed traffic signal would not restrict the progressive movement of traffic.

Is the number of adequate gaps in the major crossing traffic steam during the primary crossing period less than the number of minutes in that crossing period?

N/A

Do 20 or more students cross at this location during the highest crossing hour? N/A

Warrant 5 Satisfied? N/A

#### Warrant 6: Coordinated Signal System

This warrant is intended for application where installation of a traffic signal would help to provide proper platooning of vehicles and therefore provide progressive movement in a coordinated signal system.

Are any adjacent traffic signals located so far away that they do not provide a necessary degree of platooning and/or progressive operation?

No

Warrant 6 Satisfied? No

#### Warrant 7: Crash Experience

This warrant is intended for application where the severity and frequency of crashes are the principal reasons to consider installing a traffic control signal

Have adequate trials of alternatives failed to reduce the crash frequency?

N/A

Have 5 or more crashes, of types susceptible to correction by a signal, occurred within a 12-month period?

Condition A - Minimum Vehicular Volume (100% Columns): No

Is Condition B criterion met for 80% columns of Warrant 1 met?

Interruption

Are observed pedestrian volumes equal to or greater than 80% of what is required for Warrant 4?

Compination of Conditions A & D (80% Columns):

Warrant 7 Satisfied?

Agency/Company: Sanderson Stewart

Date: 9/27/2022

Project Number: 18008 33

Project Number: 18098.33

Project Description: Babcock and Mendenhall Safety Improvements

Jurisdiction: City of Bozeman/MDT

Major Street Speed Limit: 25 mph

Major Street (Approach Lanes):

Minor Street (Approach Lanes):

Analysis Year/Case:

Babcock Street (2 lane)

Black Avenue (1 lane)

Existing (2022)

#### Warrant 8: Roadway Network

This warrant is intended for application where installation of a traffic signal could be justified in order to encourage concentration and organization of traffic flow on a roadway network

Do two or more of the intersecting routes at this location have at least one of the following characteristics:

- A. It is part of the street or highway system that serves as the principal roadway network for through traffic flow; or
- B. It includes rural or suburban highways outside, entering, or traversing a City; or
- C. It appears as a major route on an official plan.

No

Does this intersection have an existing or immediately projected total entering volume of a least 1000 vehicles during a weekday typical peak hour and have a 5-year projected traffic volume that meets one or more of Warrants 1, 2, and 3 during an average weekday?

No

Does this intersection have an existing or immediately projected total entering volume of at least 1000 vph for each of any 5 hours of a Saturday or Sunday?

N/A

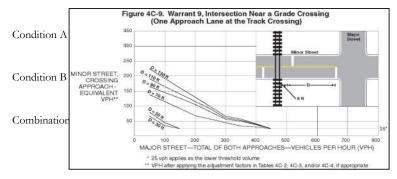
Warrant 8 Satisfied?

#### Warrant 9: Intersection Near a Grade Crossing

This warrant is intended for application where none of the conditions described in the other eight traffic signal warrants are met, but the proximity to the intersection of a grade crossing on an intersection approach controlled by a STOP or YIELD sign is the principal reason to consider installing a traffic signal.

Does a grade crossing exist on an approach controlled by a STOP or YIELD sign whereby the center of the track nearest to the intersection is within 140 feet of the stop or yield line?

No



During the highest traffic volume hour during which the rail traffic uses the crossing, does the plotted point representing vehicles per hour on the major street and the corresponding vehicles per hour on the minor-street approach that crosses the track fall above the applicable curve in Figure 4C-9 or 4C-10 (whichever is applicable) for the existing combination of approach lanes over the track and the distance D, which is the clear storage distance?

N/A

Warrant 9 Satisfied?

N/A





	Mendenhall -	Mendenhall -	Babcock -	Babcock -
	3rd & Grand	Bozeman & Rouse	Black & Bozeman	3rd & Grand
Date	7/11-13/2022	7/13-15/2022	7/13-15/2022	7/11-13/2022
Direction of Vehicles	Westbound	Westbound	Eastbound	Eastbound
ADT (veh)	5,148	4,355	5,774	4,497
85th Percentile Speed (mph)	30	27	28	30
Max speed (mph)	49	55	49	48
50th Percentile Speed (mph)	26	23	24	26

For Project: Mendenhall 3rd and Grand

Projects Notes/Address:
Location/Name: Merged

Report Generated: 1/19/2023 10:27:33 AM

Speed Intervals = 1 MPH
Time Intervals = Instant

Traffic Report From 7/11/2022 01:00:00 PM through 7/13/2022 01:59:59 PM

85th Percentile Speed = 30.0 MPH 85th Percentile Vehicles = 8,934 counts Max Speed = 79.0 MPH on 7/13/2022 9:59:33 AM

Total Vehicles =10,511 counts

AADT: 5148.3

Volumes - weekly vehicle counts

Time 5 Day 7 Day Average Daily 3,503 3,503 3,503 AM Peak 11:00 to 12:00 386 386 PM Peak 05:00 to 06:00 422 422

Speed

Speed Limit: 25 MPH

85th Percentile Speed: 30.0 MPH 50th Percentile Speed: 26.0 MPH

10 MPH Pace Interval: 20.0 MPH to 30.0 MPH

Average Speed: 25.4 MPH

	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday
Count over limit	1675	2569	1065	N/A	N/A	N/A	N/A
% over limit	54.0	49.6	47.7	N/A	N/A	N/A	N/A
Avg Speeder	28.8	28.8	29.7	N/A	N/A	N/A	N/A

Class Counts

 Number
 %

 VEH\_SM
 41
 0.4

 VEH\_MED
 10208
 97.1

 VEH\_LG
 262
 2.5

[ VEH\_SM = motorcycle, VEH\_MED = sedan, VEH\_LG = truck ]

85th percentile spec	·	_	Motol C-t-	Man Cara d	Barrer Corner de la	0. 0
Date/Time Ending	85th pctl(MPH)	85th pctl counts	Total Cnts	Max Speed	Avg Speeder	% Speeders
7/11/2022 02:00:00 PM	28.0	115	135	37	28.2	37.0%
7/11/2022 03:00:00 PM	28.0	342	402	37	28.4	41.8%
7/11/2022 04:00:00 PM	29.0	371	436	36	28.4	43.1%
7/11/2022 05:00:00 PM	30.0	346	407	40	28.7	55.3%
7/11/2022 06:00:00 PM	30.0	402	473	39	28.9	65.1%
7/11/2022 07:00:00 PM	30.0	278	327	43	28.9	60.9%
7/11/2022 08:00:00 PM	29.0	246	289	37	28.4	51.9%
7/11/2022 09:00:00 PM	32.0	188	221	4 4	30.1	60.2%
7/11/2022 10:00:00 PM	30.0	179	211	37	28.9	60.2%
7/11/2022 11:00:00 PM	31.0	108	127	39	29.4	64.6%
7/12/2022 12:00:00 AM	31.0	65	76	35	29.5	59.2%
7/12/2022 01:00:00 AM	32.0	26	31	37	30.1	71.0%
7/12/2022 02:00:00 AM	31.0	19	22	42	29.8	68.2%
7/12/2022 03:00:00 AM	33.0	6	7	33	30.0	100.0%
7/12/2022 04:00:00 AM	28.0	4	5	32	29.3	60.0%
7/12/2022 05:00:00 AM	29.0	7	8	39	29.4	87.5%
7/12/2022 06:00:00 AM	32.0	29	34	44	29.9	76.5%
7/12/2022 07:00:00 AM	32.0	59	69	39	29.6	69.6%
7/12/2022 08:00:00 AM	30.0	150	177	43	29.3	70.1%
7/12/2022 09:00:00 AM	30.0	206	242	46	28.8	57.4%
7/12/2022 10:00:00 AM	29.0	228	268	55	28.9	42.9%
7/12/2022 11:00:00 AM	28.0	253	298	51	28.9	37.9%
7/12/2022 12:00:00 PM	27.0	298	351	40	28.1	34.2%
7/12/2022 01:00:00 PM	28.0	329	387	56	28.5	33.9%
7/12/2022 02:00:00 PM	29.0	314	369	41	28.5	38.5%
7/12/2022 03:00:00 PM	28.0	304	358	59	29.0	42.7%
7/12/2022 04:00:00 PM	29.0	314	369	59	28.7	42.0%
7/12/2022 05:00:00 PM	29.0	320	376	47	28.6	45.2%
7/12/2022 06:00:00 PM	30.0	316	372	38	28.5	61.6%
7/12/2022 07:00:00 PM	30.0	300	353	42	28.7	65.2%
7/12/2022 08:00:00 PM	30.0	257	302	41	28.8	53.0%
7/12/2022 09:00:00 PM	29.0	267	314	44	28.7	58.0%
7/12/2022 10:00:00 PM	31.0	200	235	40	29.1	56.2%
7/12/2022 11:00:00 PM	30.0	129	152	40	28.8	65.8%
7/13/2022 12:00:00 AM	30.0	66	78	40	29.1	60.3%
7/13/2022 01:00:00 AM	31.0	29	34	36	29.6	76.5%
7/13/2022 02:00:00 AM	30.0	15	18	40	29.7	61.1%
7/13/2022 03:00:00 AM	30.0	8	9	31	29.4	77.8%
7/13/2022 04:00:00 AM	32.0	5	6	40	32.4	83.3%
7/13/2022 05:00:00 AM	28.0	8	10	30	28.2	60.0%
7/13/2022 06:00:00 AM	31.0	20	24	36	29.8	58.3%
7/13/2022 07:00:00 AM	33.0	60	70	38	30.1	67.1%
7/13/2022 08:00:00 AM	32.0	142	167	72	30.1	71.9%
7/13/2022 09:00:00 AM	30.0	184	217	49	28.9	59.9%
7/13/2022 10:00:00 AM	29.0	212	250	79	30.6	44.4%
7/13/2022 10:00:00 AM	29.0	248	292	66	29.3	45.9%
7/13/2022 11:00:00 AM 7/13/2022 12:00:00 PM	29.0	359	422	74	29.8	42.9%
7/13/2022 12:00:00 PM 7/13/2022 01:00:00 PM	29.0	369	434	62	29.4	40.8%
7/13/2022 01:00:00 PM 7/13/2022 02:00:00 PM	29.0	237	279	70	30.2	34.8%
// 13/2022 02.00.00 FM	∠ J • U	231	213	70	30.2	J4.0°

85th percentile speeds, counts and total counts by day:								
Date/Time Ending	85th pctl(MPH)	85th pctl cnts	Total Cnts	Max Speed	Avg Speeder	% Speeders		
7/12/2022 12:00:00 AM	30.0	2638	3104	44	28.8	54.0%		
7/13/2022 12:00:00 AM	29.0	4400	5176	59	28.8	49.6%		
7/13/2022 01:59:59 PM	30.0	1896	2231	79	29.7	47.7%		

For Project: Mendenhall Bozeman and Rouse

Projects Notes/Address: Location/Name: Merged

Report Generated: 1/19/2023 10:31:11 AM

Speed Intervals = 1 MPH
Time Intervals = Instant

Traffic Report From 7/13/2022 02:00:00 PM through 7/15/2022 02:59:59 PM

85th Percentile Speed = 27.0 MPH 85th Percentile Vehicles = 7,559 counts Max Speed = 55.0 MPH on 7/13/2022 9:04:08 PM

Total Vehicles =8,893 counts

AADT: 4355.8

Volumes - weekly vehicle counts

Time 5 Day 7 Day Average Daily 2,964 2,964 2,964 AM Peak 11:00 to 12:00 308 308 PM Peak 05:00 to 06:00 451

Speed

Speed Limit: 25 MPH

85th Percentile Speed: 27.0 MPH 50th Percentile Speed: 23.0 MPH

10 MPH Pace Interval: 18.0 MPH to 28.0 MPH

Average Speed: 22.9 MPH

Monday Wednesday Thursday Friday Saturday Sunday Tuesday N/ACount over limit N/AN/A460 1099 586 N/A% over limit N/AN/A23.3 22.0 30.5 N/AN/AAvg Speeder N/AN/A27.7 27.9 27.8 N/AN/A

Class Counts

 Number
 %

 VEH\_SM
 88
 1.0

 VEH\_MED
 8595
 96.6

 VEH\_LG
 210
 2.4

85th percentile speeds, counts and total counts by day:

27.0

27.0

27.0

85th pctl(MPH)

85th pctl cnts

1677

4249

1633

Date/Time Ending

7/14/2022 12:00:00 AM

7/15/2022 12:00:00 AM

7/15/2022 02:59:59 PM

[ VEH\_SM = motorcycle, VEH\_MED = sedan, VEH\_LG = truck ]

85th percentile spee						^
Date/Time Ending	85th pctl(MPH)	85th pctl counts	Total Cnts	Max Speed	Avg Speeder	% Speeders
7/13/2022 03:00:00 PM	27.0	139	163	33	27.6	23.3%
7/13/2022 04:00:00 PM	27.0	285	335	33	27.3	23.9%
7/13/2022 05:00:00 PM	27.0	269	317	33	27.8	24.6%
7/13/2022 06:00:00 PM	26.0	311	366	34	27.8	24.3%
7/13/2022 07:00:00 PM	26.0	219	258	37	27.6	24.0%
7/13/2022 08:00:00 PM	27.0	143	168	34	27.8	25.6%
7/13/2022 09:00:00 PM	26.0	139	163	35	28.0	22.1%
7/13/2022 10:00:00 PM	26.0	80	94	55	29.8	16.0%
7/13/2022 11:00:00 PM	26.0	58	68	36	27.8	19.1%
7/14/2022 12:00:00 AM	25.0	35	41	30	27.3	14.6%
7/14/2022 01:00:00 AM	26.0	22	26	30	27.2	19.2%
7/14/2022 02:00:00 AM	28.0	8	9	29	28.5	22.2%
7/14/2022 03:00:00 AM	26.0	9	11	29	27.7	27.3%
7/14/2022 04:00:00 AM	23.0	3	4	28	28.0	25.0%
7/14/2022 05:00:00 AM	26.0	6	7	28	26.4	71.4%
7/14/2022 06:00:00 AM	26.0	22	26	31	28.2	19.2%
7/14/2022 07:00:00 AM	29.0	36	42	33	28.5	40.5%
7/14/2022 08:00:00 AM	27.0	109	128	32	27.8	35.2%
7/14/2022 09:00:00 AM	28.0	173	204	37	27.9	42.6%
7/14/2022 10:00:00 AM	27.0	170	200	36	28.1	32.0%
7/14/2022 11:00:00 AM	27.0	185	218	34	27.8	30.3%
7/14/2022 12:00:00 PM	27.0	252	297	34	27.7	28.6%
7/14/2022 01:00:00 PM	27.0	288	339	42	28.2	23.0%
7/14/2022 02:00:00 PM	27.0	278	327	37	27.7	25.7%
7/14/2022 03:00:00 PM	26.0	304	358	36	27.6	23.7%
7/14/2022 04:00:00 PM	26.0	279	328	36	27.7	18.3%
7/14/2022 05:00:00 PM	26.0	416	489	40	28.5	17.6%
7/14/2022 06:00:00 PM	26.0	456	537	39	27.9	20.9%
7/14/2022 07:00:00 PM	26.0	334	393	36	27.7	17.3%
7/14/2022 08:00:00 PM	25.0	314	370	38	28.6	12.2%
7/14/2022 09:00:00 PM	25.0	247	291	35	28.1	12.7%
7/14/2022 10:00:00 PM	26.0	196	230	43	28.5	15.7%
7/14/2022 11:00:00 PM	25.0	86	101	32	27.6	13.9%
7/15/2022 12:00:00 AM	25.0	57	67	30	27.2	13.4%
7/15/2022 01:00:00 AM	25.0	37	43	30	29.0	14.0%
7/15/2022 02:00:00 AM	25.0	26	30	45	32.3	13.3%
7/15/2022 03:00:00 AM	25.0	16	19	33	28.3	15.8%
7/15/2022 04:00:00 AM	25.0	3	4	26	26.0	25.0%
7/15/2022 05:00:00 AM	29.0	4	5	30	29.0	60.0%
7/15/2022 06:00:00 AM	27.0	20	24	29	27.7	29.2%
7/15/2022 07:00:00 AM	27.0	40	47	39	28.6	34.0%
7/15/2022 08:00:00 AM	28.0	71	84	34	27.5	46.4%
7/15/2022 09:00:00 AM	28.0	105	123	32	27.9	39.8%
7/15/2022 10:00:00 AM	28.0	186	219	33	27.7	35.2%
7/15/2022 11:00:00 AM	27.0	230	270	33	27.6	32.6%
7/15/2022 12:00:00 PM	27.0	272	320	39	27.7	27.8%
7/15/2022 01:00:00 PM	27.0	277	326	42	28.1	25.8%
7/15/2022 02:00:00 PM	27.0	235	276	33	27.8	29.0%
7/15/2022 02:00:00 PM	27.0	112	132	33	27.7	30.3%
,,10,2022 00.00 IM	2 / • V	± ± €	102	55	21.1	JU • J 0

Total Cnts

1973

4999

1921

Max Speed

55

43

45

% Speeders

23.3%

30.5%

Avg Speeder

27.7

27.9

27.8

For Project: Babcock Black and Bozeman

Projects Notes/Address:
Location/Name: Merged

Report Generated: 1/19/2023 10:05:20 AM

Speed Intervals = 1 MPH
Time Intervals = Instant

Traffic Report From 7/13/2022 03:00:00 PM through 7/15/2022 03:59:59 PM

85th Percentile Speed = 28.0 MPH 85th Percentile Vehicles = 10,021 counts Max Speed = 49.0 MPH on 7/15/2022 2:42:33 PM

Total Vehicles =11,789 counts

AADT: 5774.2

Volumes - weekly vehicle counts

 Time
 5 Day
 7 Day

 Average Daily
 3,929
 3,929

 AM Peak
 11:00 to 12:00
 425

 PM Peak
 05:00 to 06:00
 523

Speed

Speed Limit: 25 MPH

85th Percentile Speed: 28.0 MPH 50th Percentile Speed: 24.0 MPH

10 MPH Pace Interval: 18.0 MPH to 28.0 MPH

Average Speed: 23.6 MPH

Thursday Monday Wednesday Friday Saturday Sunday Tuesday Count over limit N/AN/AN/A747 1775 1200 N/A% over limit N/AN/A40.0 28.0 33.5 N/AN/A28.5 Avg Speeder N/AN/A28.3 28.5 N/AN/A

Class Counts

 Number
 %

 VEH\_SM
 331
 2.8

 VEH\_MED
 11251
 95.4

 VEH\_LG
 207
 1.8

[ VEH\_SM = motorcycle, VEH\_MED = sedan, VEH\_LG = truck ]

		al counts by hour:	m-4:3 0 1	v. ~ .		^ ~
Date/Time Ending	85th pctl (MPH)	85th pctl counts	Total Cnts	Max Speed	Avg Speeder	% Speeders
7/13/2022 04:00:00 PM	28.0	181	213	45	28.5	29.1%
7/13/2022 05:00:00 PM	29.0	314	369	45	28.4	39.6%
7/13/2022 06:00:00 PM	28.0	368	433	37	28.3	33.3%
7/13/2022 07:00:00 PM	28.0	264	311	45	28.3	41.5%
7/13/2022 08:00:00 PM	30.0	190	224	4 4	28.9	51.3%
7/13/2022 09:00:00 PM	28.0	125	147	35	28.0	42.9%
7/13/2022 10:00:00 PM	29.0	81	95	34	28.4	46.3%
7/13/2022 11:00:00 PM	30.0	45	53	39	29.1	52.8%
7/14/2022 12:00:00 AM	31.0	20	23	35	29.3	69.6%
7/14/2022 01:00:00 AM	29.0	19	22	33	28.4	54.5%
7/14/2022 02:00:00 AM	31.0	8	9	32	29.8	55.6%
7/14/2022 03:00:00 AM	21.0	4	5	25	0.0	0.0%
7/14/2022 04:00:00 AM	26.0	1	1	26	26.0	100.0%
7/14/2022 05:00:00 AM	30.0	6	7	32	29.8	57.1%
7/14/2022 06:00:00 AM	30.0	46	54	35	29.1	72.2%
7/14/2022 07:00:00 AM	32.0	106	125	39	29.6	72.0%
7/14/2022 08:00:00 AM	31.0	259	305	38	29.2	61.0%
7/14/2022 09:00:00 AM	29.0	329	387	36	28.4	53.0%
7/14/2022 10:00:00 AM	28.0	328	386	35	27.7	35.5%
7/14/2022 11:00:00 AM	27.0	241	284	38	28.2	26.8%
7/14/2022 12:00:00 PM	27.0	366	431	48	28.4	27.1%
7/14/2022 01:00:00 PM	26.0	338	398	37	28.1	17.6%
7/14/2022 02:00:00 PM	27.0	386	454	40	28.1	28.0%
7/14/2022 03:00:00 PM	27.0	364	428	34	27.8	22.7%
7/14/2022 04:00:00 PM	27.0	361	425	40	27.9	25.9%
7/14/2022 05:00:00 PM	26.0	383	451	38	27.7	18.6%
7/14/2022 06:00:00 PM	27.0	522	614	43	28.2	26.2%
7/14/2022 07:00:00 PM	25.0	428	503	35	27.9	14.7%
7/14/2022 08:00:00 PM	25.0	332	391	35	27.6	14.3%
7/14/2022 09:00:00 PM	25.0	253	298	33	27.7	14.8%
7/14/2022 10:00:00 PM	26.0	193	227	38	28.1	19.4%
7/14/2022 10:00:00 PM	27.0	69	81	34	28.3	23.5%
7/15/2022 11:00:00 PM 7/15/2022 12:00:00 AM	27.0	48	57	35	28.5	29.8%
7/15/2022 12:00:00 AM	30.0	25	29	33	28.2	55.2%
7/15/2022 02:00:00 AM	28.0	20	24	33	28.2	37.5%
7/15/2022 03:00:00 AM	30.0	13	15	34	30.0	33.3%
7/15/2022 04:00:00 AM	32.0	3	4	34	32.0	75.0%
7/15/2022 05:00:00 AM	29.0	8	9	32	28.5	66.7%
7/15/2022 06:00:00 AM	32.0	42	50	46	30.2	82.0%
7/15/2022 07:00:00 AM	31.0	88	104	37	29.4	63.5%
7/15/2022 08:00:00 AM	30.0	207	243	40	28.8	70.4%
7/15/2022 09:00:00 AM	29.0	321	378	41	28.5	51.9%
7/15/2022 10:00:00 AM	28.0	312	367	39	28.4	40.1%
7/15/2022 11:00:00 AM	27.0	326	384	36	27.9	25.3%
7/15/2022 12:00:00 PM	26.0	357	420	40	27.9	19.8%
7/15/2022 01:00:00 PM	26.0	366	430	33	27.4	18.8%
7/15/2022 02:00:00 PM	28.0	374	440	45	28.6	28.9%
7/15/2022 03:00:00 PM	26.0	367	432	49	28.6	18.3%

85th percentile speeds, counts and total counts by day:								
Date/Time Ending	85th pctl(MPH)	85th pctl cnts	Total Cnts	Max Speed	Avg Speeder	% Speeders		
7/14/2022 12:00:00 AM	29.0	1588	1868	45	28.5	40.0%		
7/15/2022 12:00:00 AM	27.0	5391	6342	48	28.3	28.0%		
7/15/2022 03:59:59 PM	28.0	3042	3579	49	28.5	33.5%		

For Project: Babcock 3rd and Grand

Projects Notes/Address: Location/Name: Merged

Report Generated: 1/19/2023 10:03:30 AM

Speed Intervals = 1 MPH
Time Intervals = Instant

Traffic Report From 7/11/2022 01:00:00 PM through 7/13/2022 01:59:59 PM

85th Percentile Speed = 30.0 MPH 85th Percentile Vehicles = 7,806 counts Max Speed = 48.0 MPH on 7/12/2022 7:00:58 PM

Total Vehicles =9,183 counts

AADT: 4497.8

Volumes - weekly vehicle counts

Time 5 Day 7 Day Average Daily 3,061 3,061 3,061 AM Peak 11:00 to 12:00 368 PM Peak 12:00 to 01:00 423

Speed

Speed Limit: 25 MPH

85th Percentile Speed: 30.0 MPH 50th Percentile Speed: 26.0 MPH

10 MPH Pace Interval: 21.0 MPH to 31.0 MPH

Average Speed: 25.5 MPH

Wednesday Thursday Monday Friday Saturday Sunday Tuesday Count over limit N/A1231 2467 1167 N/AN/AN/A% over limit 54.5 52.8 51.8 N/AN/AN/AN/AAvg Speeder 28.7 28.7 29.0 N/AN/AN/AN/A

Class Counts

 Number
 %

 VEH\_SM
 589
 6.4

 VEH\_MED
 8313
 90.5

 VEH\_LG
 281
 3.1

85th percentile speeds, counts and total counts by hour:

[ VEH\_SM = motorcycle, VEH\_MED = sedan, VEH\_LG = truck ]

open bercentire spee	ds, counts and cota	it counts by nour.				
Date/Time Ending	85th pctl(MPH)	85th pctl counts	Total Cnts	Max Speed	Avg Speeder	% Speeders
7/11/2022 02:00:00 PM	29.0	215	253	36	28.5	56.1%
7/11/2022 03:00:00 PM	30.0	278	327	42	28.6	57.2%
7/11/2022 04:00:00 PM	29.0	267	314	43	28.4	52.9%
7/11/2022 05:00:00 PM	30.0	283	333	47	28.8	52.9%
7/11/2022 06:00:00 PM	29.0	302	355	41	28.5	53.8%
7/11/2022 07:00:00 PM	31.0	182	214	40	29.0	61.2%
7/11/2022 08:00:00 PM	30.0	156	184	41	28.9	55.4%
7/11/2022 09:00:00 PM	30.0	99	117	38	29.1	52.1%
7/11/2022 10:00:00 PM	31.0	72	85	37	29.9	44.7%
7/11/2022 11:00:00 PM	29.0	44	52	43	30.0	48.1%
7/12/2022 12:00:00 AM	30.0	20	23	33	28.8	52.2%
7/12/2022 01:00:00 AM	28.0	13	15	32	28.5	40.0%
7/12/2022 02:00:00 AM	28.0	3	4	29	27.5	100.0%
7/12/2022 03:00:00 AM	26.0	4	5	28	26.5	80.0%
7/12/2022 04:00:00 AM	26.0	2	2	26	26.0	50.0%
7/12/2022 05:00:00 AM	32.0	4	5	33	29.8	80.0%
7/12/2022 06:00:00 AM	31.0	17	20	34	29.5	65.0%
7/12/2022 07:00:00 AM	33.0	52	61	39	30.0	73.8%
7/12/2022 08:00:00 AM	32.0	172	202	40	29.6	70.8%
7/12/2022 09:00:00 AM	30.0	286	336	44	29.0	60.1%
7/12/2022 10:00:00 AM	30.0	237	279	40	29.0	53.0%
7/12/2022 11:00:00 AM	30.0	274	322	45	28.9	57.8%
7/12/2022 12:00:00 PM	29.0	298	350	40	28.6	47.1%
7/12/2022 01:00:00 PM	29.0	342	402	38	28.2	47.3%
7/12/2022 02:00:00 PM	29.0	318	374	38	28.5	44.4%
7/12/2022 03:00:00 PM	29.0	307	361	38	28.5	50.4%
7/12/2022 04:00:00 PM	30.0	309	364	36	28.8	48.1%
7/12/2022 05:00:00 PM	30.0	330	388	35	28.6	52.8%
7/12/2022 06:00:00 PM	30.0	360	423	39	28.6	57.7%
7/12/2022 07:00:00 PM	29.0	243	286	35	28.4	55.9%
7/12/2022 08:00:00 PM	30.0	167	196	48	29.3	51.5%
7/12/2022 09:00:00 PM	29.0	116	136	37	28.7	44.1%
7/12/2022 10:00:00 PM	30.0	61	72	36	29.0	47.2%
7/12/2022 11:00:00 PM	29.0	36	42	36	29.3	33.3%
7/13/2022 12:00:00 AM	28.0	26	30	31	27.5	50.0%
7/13/2022 01:00:00 AM	28.0	8	9	32	28.8	44.4%
7/13/2022 02:00:00 AM	30.0	6	7	30	28.8	57.1%
7/13/2022 03:00:00 AM	23.0	2	2	23	0.0	0.0%
7/13/2022 04:00:00 AM	26.0	2	2	26	26.0	100.0%
7/13/2022 05:00:00 AM	36.0	5	6	37	31.8	83.3%
7/13/2022 06:00:00 AM	29.0	18	21	36	28.9	61.9%
7/13/2022 07:00:00 AM	32.0	55	65	36	30.1	73.8%
7/13/2022 08:00:00 AM	32.0	184	217	38	29.7	67.7%
7/13/2022 09:00:00 AM	31.0	273	321	37	29.0	65.4%
7/13/2022 10:00:00 AM	30.0	268	315	40	29.1	55.9%
7/13/2022 11:00:00 AM	29.0	258	303	41	28.6	49.5%
7/13/2022 12:00:00 PM	29.0	329	387	39	28.7	39.5%
7/13/2022 01:00:00 PM	29.0	377	4 4 4	38	28.6	41.9%
7/13/2022 02:00:00 PM	29.0	129	152	46	28.4	45.4%

85th percentile speeds, counts and total counts by day:									
Date/Time Ending	85th pctl(MPH)	85th pctl cnts	Total Cnts	Max Speed	Avg Speeder	% Speeders			
7/12/2022 12:00:00 AM	30.0	1918	2257	47	28.7	54.5%			
7/13/2022 12:00:00 AM	30.0	3974	4675	48	28.7	52.8%			
7/13/2022 01:59:59 PM	30.0	1913	2251	46	29.0	51.8%			

# ENDURING COMMUNITY DESIGN